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VERNAL UNIT  
CENTRAL UTAH PROJECT

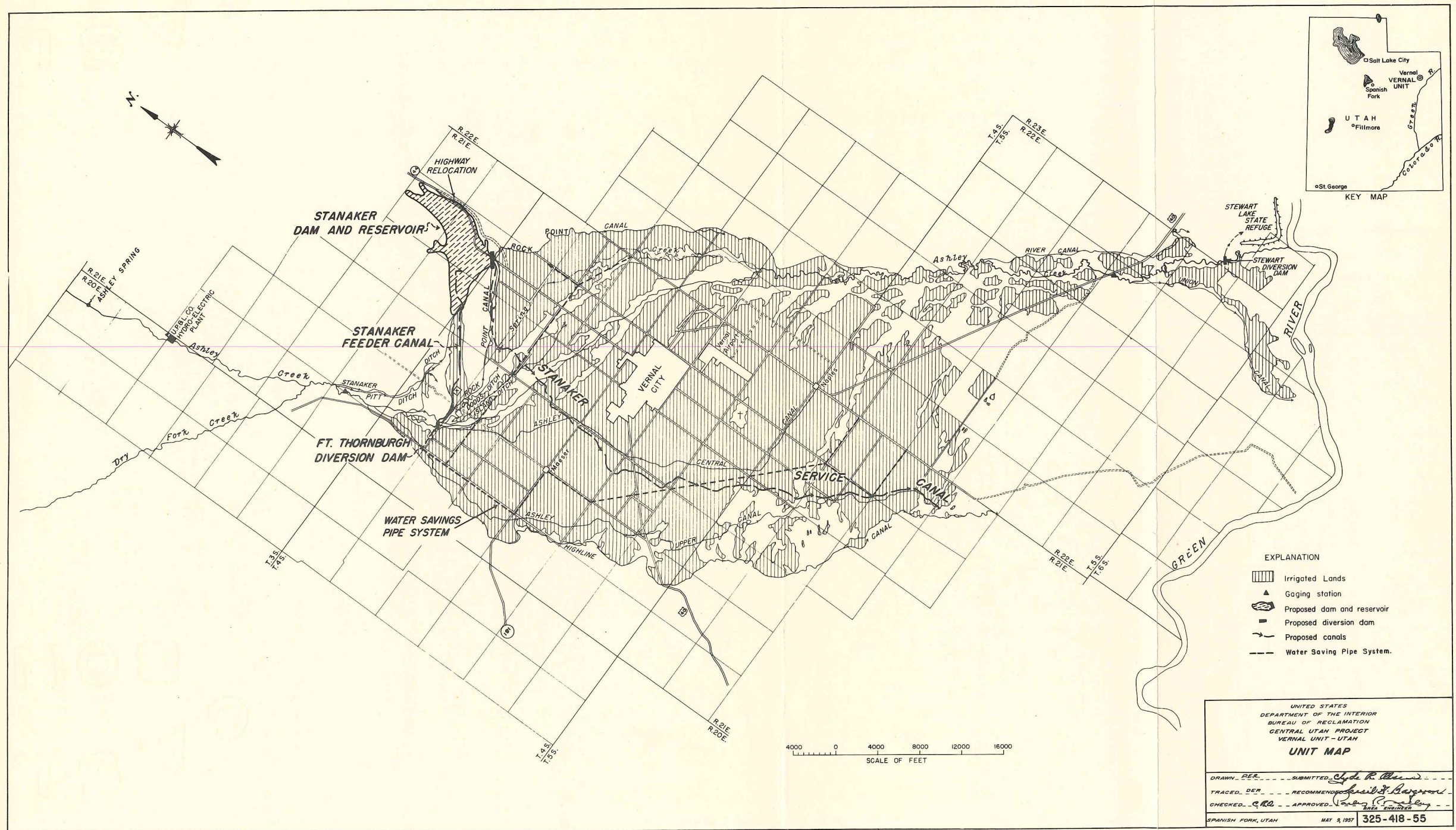
DEFINITE PLAN REPORT

APPENDIX B  
WATER SUPPLY

May 1957

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Appendixes to the Vernal Unit Definite Plan Report have been issued in four volumes with the data grouped as shown below.

APPENDIX A  
PROJECT LANDS  
LAND DRAINAGE

APPENDIX B  
WATER SUPPLY

APPENDIX C  
DESIGNS AND ESTIMATES

APPENDIX D  
AGRICULTURAL ECONOMY  
FINANCIAL ANALYSIS

## SUMMARY SHEETS

### Vernal Unit--Central Utah Project

**LOCATION:** Northeast Utah in Ashley Valley of the Uinta Basin, approximately centered by Vernal, Utah.

**AUTHORIZED:** Initial phase of the Central Utah project, including the Vernal unit, authorized as a participating project with the Colorado River Storage project by the Act of April 11 1956 (70 Stat. 105).

#### PLAN

Through storage regulation and water exchanges, the Vernal unit will provide supplemental irrigation water for 14,781 acres of land and 1,500 acre-feet of water annually to supplement the municipal supplies of Vernal, Naples, and Maeser. The unit will also provide benefits to fish and wildlife and recreation. Excess flows of Ashley Creek will be diverted at the Ft. Thornburgh Diversion Dam into the Stanaker Feeder Canal and conveyed to the Stanaker Reservoir. Water stored in the reservoir will be released into the Stanaker Service Canal and delivered to existing irrigation canals and ditches. The water will in part replace Ashley Creek water, including releases from upstream reservoirs. Some of the replaced water will be used on lands above the Stanaker Service Canal and some will be diverted from Ashley Spring on Ashley Creek into the municipal pipeline. Land drains will be provided as needed and some sections of existing canals will be lined to prevent seepage. A pipe system will be constructed for stock-watering purposes during the nonirrigation season to save for unit storage and use water now lost through open canals. Recreational and fishing attractions will be provided at Stanaker Reservoir. Small tracts of land distributed among the unit area will be acquired and developed for upland game, and a pump and pipeline will be installed to deliver water from Green River to the Stewart Lake State Refuge. Repayment of reimbursable construction costs will be completed in 50 years, following a 3-year development period. Irrigation costs that are beyond the repayment ability of the irrigators will be paid from the Upper Colorado River Basin Fund.



## SUMMARY SHEETS (Continued)

### CONSTRUCTION COSTS

1/\$6,874,000

Stanaker Dam and Reservoir . . . . .	\$3,870,000
Ft. Thornburgh Diversion Dam . . . . .	200,000
Stanaker Service Canal . . . . .	1,060,000
Stanaker Feeder Canal . . . . .	570,000
Water Savings pipe system . . . . .	340,000
Stanaker Canal laterals . . . . .	40,000
Vernal area drainage system . . . . .	675,000
Recreation . . . . .	92,000
Fish and wildlife . . . . .	27,000

1/ Estimated at January 1957 prices.

### BENEFITS, ALLOCATIONS, AND REPAYMENT

Unit purpose	Benefits (annual)	Allocations (tentative)	
		Construction costs	Annual O.M. & R. costs
Irrigation . . . . .	\$253,500	1/\$6,154,000	\$12,700
Municipal water . . . . .	23,800	2/619,000	1,800
Recreation . . . . .	14,200	3/92,000	7,100
Fish and wildlife . . . . .	13,600	3/27,000	1,200
Total . . . . .	305,100	2/6,892,000	22,800

1/ \$1,500,000 will be repaid by Vernal unit irrigators through the Upper Colorado River Basin Fund and the remaining \$4,654,000 will be paid from other revenues in the basin fund apportioned to Utah.

2/ Includes \$18,000 in interest during construction.

3/ Nonreimbursable

#### Average annual water costs per acre-foot

	Construction repayment	O.M. & R. costs
Irrigation water . . . . .	\$1.65	\$0.70
Municipal water . . . . .	1/	1.20

1/ Municipal water payment will increase from \$12.00 per acre-foot during first 10 years to \$22.13 during last 10 years of 50-year repayment period.

### BENEFIT-COST RATIO

1.44 to 1

### REPAYMENT ORGANIZATION

The Uintah Water Conservancy District has been organized in accordance with Utah State law and will contract with the United States for the repayment of irrigation and municipal water costs.

## SUMMARY SHEETS (Continued)

### IRRIGATION

<u>Irrigable area furnished supplemental water</u>		<u>Acres</u>
Class 1 . . . . .		3,286
Class 2 . . . . .		5,357
Class 3 . . . . .		5,801
Unclassified (town site). . . . .		337
Total . . . . .		14,781
Elevation of farm lands (avg. feet msl) . . . . .		5,300
Frost-free period (avg. days annually). . . . .		119
Effective precipitation (avg. inches annually). . . . .		3
Diversion requirement (avg. ac.-ft. annually). . . . .		51,700
Increase in water supply (avg. ac.-ft. annually). . . . .		18,000
Increased depletion of Colorado River from unit operation (avg. ac.-ft. annually) . . . . .		11,800

### UNIT WORKS

#### Stanaker Dam

Located on offstream Stanaker Draw, 3.5 miles north of Vernal.

Type . . . . .	rolled earth-fill
Height above ground . . . . .	140 ft.
Height above foundation . . . . .	145 ft.
Volume of embankment. . . . .	1,820,000 cu. yds.
Spillway capacity . . . . .	(emergency only)
Outlet capacity (at res. elev. 5,472) . . . . .	300 sec.-ft.

#### Stanaker Reservoir

Elevation at normal water surface (37,560 ac.-ft.). . . . .	5,516.2 ft.
Active storage capacity . . . . .	33,200 ac.-ft.
Inactive storage capacity . . . . .	4,360 ac.-ft.
Total storage capacity. . . . .	37,560 ac.-ft.
Reservoir surcharge capacity above normal water surface elevation. . . . .	2,170 ac.-ft.

#### Stanaker Feeder Canal

Length. . . . .	3.1 miles
Capacity. . . . .	400 sec.-ft.

#### Stanaker Service Canal

Length. . . . .	11.8 miles
Capacity at head. . . . .	300 sec.-ft.

#### Water Savings Pipe System

Length. . . . .	17.3 miles
Capacity at head. . . . .	289 gal. per min.



## SUMMARY SHEETS (Continued)

### HYDROLOGY

#### Ashley Creek at "Sign of the Maine" gage

Drainage area. . . . .	241 sq. mi.
Period of record . . . . .	1940-56
Average runoff, 1940-56 . . . . .	92,800 ac.-ft.
Maximum annual runoff. . . . .	142,300 ac.-ft.
Minimum annual runoff. . . . .	52,400 ac.-ft.
Maximum daily discharge of record. . . . .	2,650 sec.-ft.
Minimum daily discharge of record. . . . .	14 sec.-ft.

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## WATER SUPPLY

### CHAPTER I

#### INTRODUCTION

The Vernal Unit of the Central Utah project, located in northeastern Utah, includes most of the irrigable lands in Ashley Valley. Settlement of the valley began in 1873 with the establishment of a ranch on Ashley Creek north of the present site of Maeser. In the following years other ranchers made their homes in the valley, constructed ditches, and began irrigating large tracts of land from Ashley Creek. By 1900 most of the irrigable lands in the valley had been placed under production. Since 1900 additional small areas have been developed primarily by extensions of the canal systems.

#### Unit area

The unit area, consisting of some 56 square miles, covers the major part of Ashley Valley. Included in the area as shown in the frontispiece drawing are the communities of Vernal, Maeser, and Naples, Utah.

There are seven major canal companies serving the project area diverting from Ashley Creek. These are the Stanaker, Island, Dodds, Rock Point, Ashley Upper, Ashley Central, and Highline. The Colton Ditch is served from the Ashley Upper Canal and the Hardy Ditch is served from the Ashley Central Canal. In addition to the canals diverting primary water, the Union and River Canals divert from Ashley Creek in the lower part of the valley. There are several other minor water users having rights on some drainage areas in the unit area between the upper seven major canal and lower two major canal diversions.

A detailed land classification was made of the unit area in 1956 which is discussed in the Project Lands Appendix. A summary of this classification as applies to water supply giving the area under each of the major canals broken down as to areas above and below the proposed Stanaker Service Canal is given in Table 24.

#### Present Development and Practices

The natural flows of Ashley Creek, supplemented by limited storage, are currently used for irrigation, municipal use, and power production. By means of several diversion dams, most of the flow of Ashley Creek is taken into the forementioned nine canals and conveyed to the irrigated lands. Four of the canals, Highline, Ashley Upper, Ashley Central, and the Union, divert from the right bank of the stream and serve lands lying south and west of Ashley Creek. The other canals, Stanaker, Rock Point, Dodds, Island, and River, divert from the left bank of the stream and

serve the area north and east of the creek. Not including River and Union, these canals have a combined capacity of 808 cfs as shown in Table 1.

Table 1  
Present capacities of canals<sup>1/</sup>

<u>Canal</u>	<u>Capacity (cfs)</u>
Stanaker Ditch	15
Highline	2/182
Ashley Upper	315
Ashley Central	197
Rock Point	67
Island	26
Dodds	6
Total	<u>808</u>

<sup>1/</sup> Taken from highest flow measured into the canals for the period 1951-1955.

<sup>2/</sup> Highline Canal has a water right of 182 c.f.s. However, the highest flow measured through the canal was 96 c.f.s.

The Stanaker Canal diverts from the creek above the "Sign of the Maine" gage, all others divert below the gage as shown on Drawing No. 325-418-56.

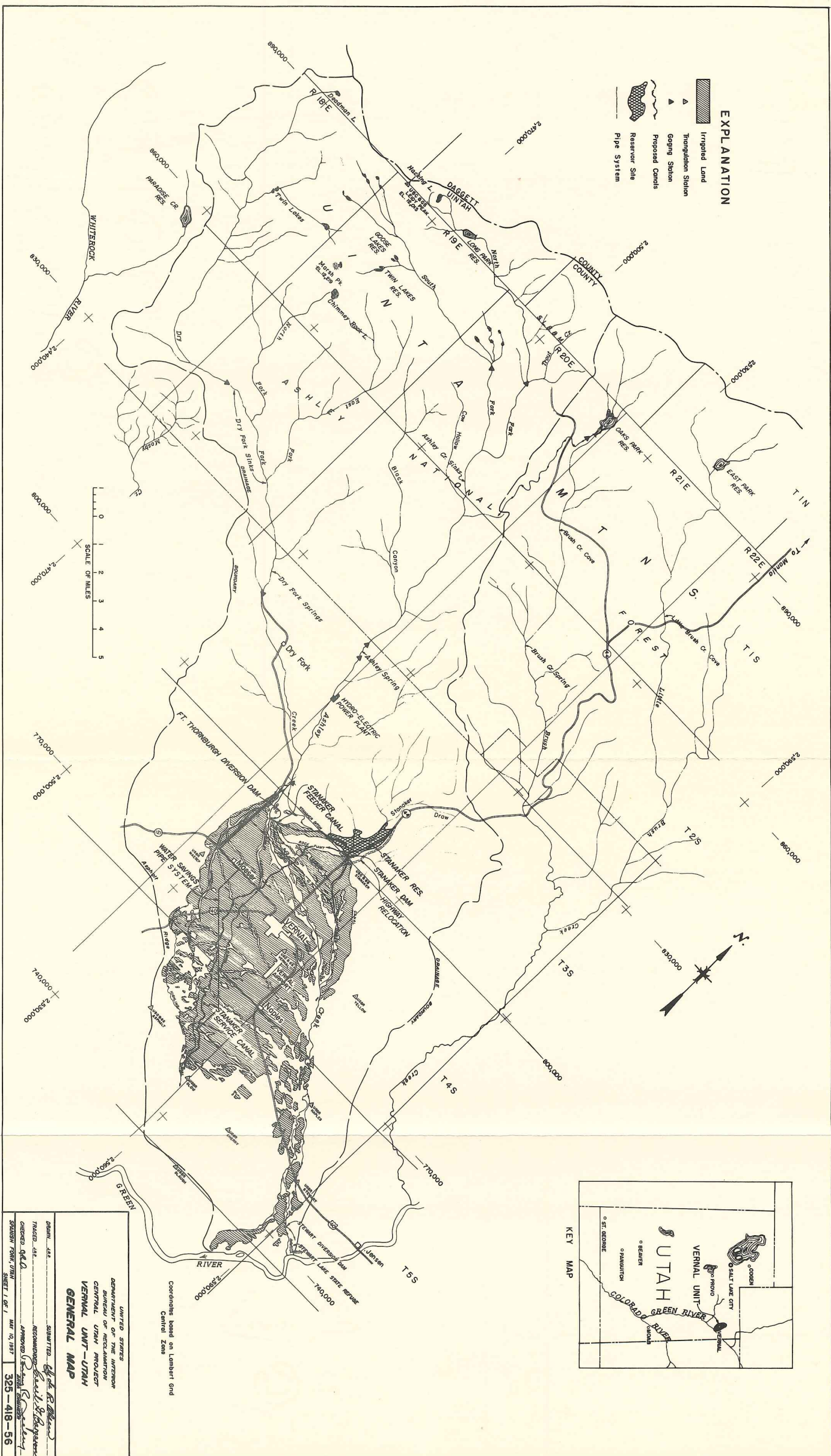
Municipal water for Vernal and the surrounding area is diverted from the Ashley Creek Springs above all irrigation diversions. Other small ditches diverting from the creek serve limited areas adjacent thereto.

A small hydroelectric plant 9 miles northwest of Vernal uses the flow of Ashley Creek to produce electrical energy for the Vernal area. The plant is situated above all diversions and does not interfere with the use of water for irrigation.

Supplemental water is provided from storage at Oaks Park Reservoir on Brush Creek, Long Park Reservoir, and a group of small glacial lakes on Ashley Creek with a combined capacity of 6,870 acre-feet. The storage water is released during the late summer months but when combined with the natural flow of Ashley Creek the total is insufficient to supply adequately the needs of presently irrigated lands.

#### Unit Plan

The unit plan contemplates the construction of an off-stream reservoir in Stanaker Draw, a feeder canal leading from Ashley Creek to the Stanaker Reservoir, and a service canal from the reservoir to Vernal Unit land. Spring flows during the high runoff period that can not be beneficially used on existing irrigated land would be diverted to Stanaker





Reservoir. Winter flows currently used for livestock watering purposes will also be diverted to the reservoir. In exchange a water savings pipe system will replace this present wasteful practice during the nonirrigation season.

Drainage facilities will be provided where necessary along with whatever rehabilitation of existing canals is required for Vernal unit operation.

Through storage regulation and water exchanges, the area's needs for increased municipal water and late season irrigation water would be supplied. The project would also provide protection of fish and wildlife, development of new recreational areas, and incidental control of floods.

Ft. Thornburgh Diversion Dam would be located on Ashley Creek at the head of the Stanaker Feeder Canal. Stanaker Feeder Canal will have a capacity of 400 c.f.s. Stanaker Reservoir, with a capacity of 37,560 acre-feet, will be formed by an earth fill dam at the mouth of Stanaker Draw about  $3\frac{1}{2}$  miles north of Vernal. The reservoir will have 33,200 acre-feet of active capacity. A dead storage pool of 3,266 acre-feet would be utilized for fish and wildlife, and an additional 1,094 acre-feet of inactive storage above the sill of the outlet works would be reserved to provide head on the outlet works during low reservoir elevation.

## WATER SUPPLY

### CHAPTER II

#### WATER RESOURCES

The Ashley Creek drainage area above irrigation diversions at the head of Ashley Valley is 238 square miles. The watershed ranges from elevation 5,700 feet to 12,000 feet, and is characterized by glaciated mountain slopes, steep canyons, a relatively impervious bed rock, and comparatively shallow soil mantle which provides very little ground water storage. As a result the runoff is rapid, contributing to wide seasonal fluctuations in stream flow. Runoff reaches its high stage of 200 to 2,650 second-feet in May and June and falls off rapidly to a flow of 14 to 300 second-feet in late summer.

#### Runoff Records

Streamflow records are available at nine gaging stations maintained by the Geological Survey. Four of these stations are significant in regard to Vernal unit water supply: Ashley Creek near Vernal, Ashley Creek at Sign of the Maine, Ashley Creek near Jensen, and Oaks Park Canal near Vernal. Table No. 2 is a summary of pertinent data for each of the nine stations which are shown on Drawing No. 325-418-56.

#### Ashley Creek at "Sign of the Maine"

The station is located 4-3/4 miles northwest of Vernal which is above all diversions to the unit area except for municipal water for Vernal area and irrigation water for Stanaker Canal. In addition to the natural stream flow the gage also reflects the releases from Oaks Park Reservoir. Elevation of the station is 5,750 and the drainage area covers 241 square miles. The gage was installed in June 1939 approximately 1 mile above the Ft. Thornburgh diversion dam and has been operated continuously to the present time. Since 1941 storage releases from Oaks Park Reservoir have been conveyed through the Oaks Park Canal and emptied into Ashley Creek above the "Sign of the Maine" gage. These releases are included in the recorded flows at the gage. Runoff at the "Sign of the Maine" gage for years of missing records during the period of study (1925-39) were estimated by correlation with runoff of "Ashley Creek near Vernal" after first deducting the Oaks Park Reservoir releases from the records of both stations. Table No. 3 gives the monthly summary of the recorded flow at the "Sign of the Maine" station for the period of 1940 through 1956. Table No. 4 gives the estimated and recorded flow less the Oaks Park Reservoir releases at the "Sign of the Maine" gage and is the water supply available for unit use.

Operation of upstream storage on Ashley Creek dates from 1919 and the operation of Oaks Park Reservoir began November 31, 1939. Because

Table 2  
Stream gaging stations on Ashley Creek

Gaging station	Location	Drain- age area square miles	Available records	Average water year runoff for period of record (acre-feet)
Ashley Creek near Vernal, Utah (below springs)	4.5 miles up- stream from Dry Fork and 10 miles northwest of Vernal.	101	Oct. 1911 to date. Frag- mentary rec- ords from 1911-18 at this and other locations	75,760
Ashley Creek at "Sign of the Maine" near Vernal, Utah	.75 mile down stream from Dry Fork and 4.75 miles from Vernal.	241	June 1939 to date	92,600
Ashley Creek above springs near Vernal, Utah	5 miles upstream from Dry Fork and 10.5 miles north- west of Vernal.	100	June 1941 to Sept. 1945	49,530
Ashley Creek below Trout Creek, near Vernal, Utah	3 miles upstream from South Fork and 21 miles north- west of Vernal.	27	October 1943 to date	17,440
South Fork of Ashley Creek near Vernal, Utah.	3.25 miles up- stream from mouth and 21 miles north- west of Vernal	20	October 1943 to date	13,630
Oaks Park Canal near Vernal, Utah	1,000 feet down stream from Oaks Park Dam	--	July 1941 to date	4,800
Ashley Creek near Jensen, Utah	3 miles upstream from mouth of U.S. Highway 40 bridge	386	October 1946 to date	
Dry Fork above Sinks near Dry Fork, Utah	14 miles upstream from mouth	48	March 1939 to date	27,670
Dry Fork below springs near Dry Fork, Utah	6 miles upstream from mouth	102	May 1941 to Sept. 1945	29,330

Table No. 3 1/  
Runoff of Ashley Creek at "Sign of the Maine" near Vernal  
Including Diversions of Oaks Park Canal from Brush Creek

Unit: 1000 a.f.

Year	Oct.	Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Total
1939										3.2	2.0	5.8	
1940	5.4	3.4	2.4	1.8	1.4	1.5	4.3	21.2	6.1	3.7	1.7	2.4	55.3
41	4.5	3.3	2.4	2.0	1.6	1.6	1.7	45.2	40.3	11.9	6.7	7.1	128.3
42	11.3	6.9	4.4	2.9	2.1	2.2	6.1	29.9	41.0	10.5	5.9	4.6	127.8
43	3.3	2.4	2.1	1.8	1.5	1.6	9.3	24.7	16.7	8.7	5.6	3.2	80.9
44	2.4	2.0	1.8	1.5	1.2	1.4	1.8	33.7	65.4	18.6	8.3	4.2	142.3
1945	2.8	2.2	1.8	1.7	1.5	1.6	1.5	19.2	24.1	10.9	7.2	5.3	79.8
46	3.8	2.5	2.0	1.7	1.4	1.4	6.8	10.6	7.9	4.5	3.2	3.3	49.1
47	3.2	3.2	2.9	2.4	1.7	2.1	3.4	52.1	34.9	14.8	7.8	5.5	134.0
48	4.3	2.6	2.3	1.9	1.5	1.6	2.0	32.5	17.9	6.9	5.3	2.3	81.1
49	2.0	1.7	1.6	1.4	1.2	1.3	2.5	30.5	41.1	10.8	7.0	3.5	104.6
1950	3.6	2.8	2.2	2.0	1.7	2.0	4.5	36.0	37.7	13.7	7.8	4.3	118.3
51	3.2	2.2	2.0	1.8	1.4	1.5	1.3	21.7	22.4	7.5	6.5	4.3	75.8
52	3.6	2.3	1.9	1.8	1.5	1.6	4.1	46.6	38.2	11.5	13.0	6.1	132.2
53	4.4	2.9	2.3	2.2	1.8	2.0	2.1	88.8	22.6	8.4	5.3	2.3	65.1
54	1.8	2.0	2.0	1.8	1.4	1.4	3.2	23.3	8.7	6.8	3.7	2.5	58.6
1955	2.5	2.1	1.8	1.6	1.3	1.4	1.5	17.5	11.1	5.1	4.0	2.5	52.4
56	2.4	1.7	1.8	1.6	1.3	1.4	2.2	31.6	14.4	6.3	3.3	1.7	69.7
1940-1956 2/													
Total	64.5	46.2	37.7	31.9	25.5	27.6	58.3	485.1	450.5	160.6	102.3	65.1	1555.3
Mean	3.8	2.7	2.2	1.9	1.5	1.6	3.4	28.5	26.5	9.5	6.0	3.8	91.5
1941-1956 3/													
Total	59.1	42.8	35.3	30.1	24.1	26.1	54.0	463.9	444.4	156.9	100.6	62.7	1500.0
Mean	3.7	2.7	2.2	1.9	1.5	1.6	3.4	29.0	27.8	9.8	6.3	3.9	93.8

1/ USGS Records from July 1939 to present.

2/ For all full years of record. Oaks Park Canal diversions did not begin until 1941.

3/ Portion of record during which Oaks Park canal diversions were made.



Table No. 4  
Natural Runoff of Ashley Creek at Sign of the Maine<sup>1/</sup>  
(Minus Oaks Park Canal Diversions)

Year	Oct.	Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Total
1925	2.4	2.3	2.2	2.1	2.0	2.1	3.2	17.0	17.1	7.6	5.1	5.8	68.9
26	7.0	4.9	3.4	2.6	2.0	2.2	8.8	27.7	11.3	6.2	4.8	2.8	83.7
27	2.7	2.3	2.2	1.9	1.8	1.9	2.4	36.2	27.3	12.5	7.2	17.9	116.3
28	7.4	5.9	3.6	3.0	2.6	2.5	3.9	49.1	19.2	6.8	4.5	2.6	111.1
29	3.0	2.9	2.3	2.5	2.0	2.1	2.4	38.3	56.0	13.0	7.4	7.2	139.1
1930	5.5	4.3	3.3	3.0	2.3	2.4	6.4	30.5	24.0	6.7	8.2	6.1	102.7
31	5.4	3.6	3.0	2.7	2.1	2.3	2.6	9.4	4.0	2.0	1.9	1.8	40.8
32	2.1	2.1	2.1	2.0	2.0	2.0	2.0	33.5	30.1	10.7	6.0	4.4	99.0
33	3.4	2.8	2.0	1.8	1.6	1.6	1.5	9.4	24.4	4.2	3.3	2.0	58.0
34	2.0	1.8	1.7	1.6	1.4	1.5	3.5	7.2	2.0	1.6	2.6	2.3	29.2
1935	2.2	2.2	2.1	1.9	1.7	1.9	1.8	13.3	47.6	7.2	3.8	2.1	87.8
36	2.0	1.9	1.7	1.5	1.4	1.4	2.2	12.6	6.5	3.8	4.8	3.9	43.7
37	3.2	2.7	1.9	1.7	1.3	1.4	2.3	45.2	24.8	11.6	6.1	3.7	105.9
38	2.9	2.3	1.9	1.7	1.4	1.6	2.3	26.8	42.5	12.0	5.8	4.9	106.1
39	7.0	5.1	3.5	2.7	2.1	2.4	7.9	22.5	7.8	3.2	2.0	5.8	72.0
1940	5.4	3.4	2.4	1.8	1.4	1.5	4.3	21.2	6.1	3.7	1.7	2.4	55.3
41	4.5	3.3	2.4	2.0	1.6	1.6	1.7	45.2	40.3	10.5	5.8	6.3	125.2
42	11.3	6.9	4.4	2.9	2.1	2.2	6.1	29.9	41.0	9.4	3.9	2.3	122.4
43	2.7	2.4	2.1	1.8	1.5	1.6	9.3	24.7	16.7	6.1	3.6	2.1	74.6
44	2.4	2.0	1.8	1.5	1.2	1.4	1.8	33.7	65.4	17.3	5.1	2.8	136.4
1945	2.7	2.2	1.8	1.7	1.5	1.6	1.5	19.2	24.1	9.4	5.7	3.8	75.2
46	3.2	2.4	2.0	1.7	1.4	1.4	6.8	10.5	6.0	2.7	3.0	3.1	44.2
47	3.2	3.2	2.9	2.4	1.7	2.1	3.4	52.0	34.8	13.6	6.6	4.4	130.3
48	3.0	2.6	2.3	1.9	1.5	1.6	2.0	32.5	17.7	4.9	2.9	1.5	74.4
49	4.8	1.7	1.6	1.4	1.2	1.3	2.5	30.5	40.9	9.5	4.5	2.5	99.4
1950	3.3	2.8	2.2	2.0	1.7	2.0	4.5	36.0	37.7	13.1	5.2	3.1	113.6
51	2.4	2.2	2.0	1.8	1.4	1.5	1.3	21.7	22.4	5.3	5.4	3.7	71.1
52	3.0	2.3	1.9	1.8	1.5	1.6	4.1	46.6	38.2	9.6	12.5	5.4	128.5
53	3.1	2.9	2.3	2.2	1.8	2.0	2.1	8.8	22.6	5.9	2.6	1.8	58.1
54	1.8	2.0	2.0	1.8	1.4	1.4	3.2	23.3	7.7	4.6	3.0	2.2	54.4
1955	2.5	2.1	1.8	1.6	1.2	1.4	1.5	17.5	11.0	3.3	3.9	1.2	49.0
56	2.4	1.7	1.8	1.6	1.3	1.4	2.2	31.6	14.4	4.0	1.8	1.1	65.3
1941-1956 <sup>2/</sup>													
Total	53.3	42.3	35.3	30.1	24.1	26.1	54.0	463.7	440.8	129.1	75.4	48.5	1422.7
Mean	3.3	2.6	2.2	1.9	1.5	1.6	3.4	29.0	27.6	8.1	4.7	3.0	88.9
1925-1956 <sup>3/</sup>													
Total	116.9	93.2	74.6	64.6	53.1	56.9	111.5	863.6	791.6	242.0	150.7	123.0	2741.7
Mean	3.7	2.9	2.3	2.0	1.7	1.8	3.5	27.0	24.7	7.6	4.7	3.8	85.7

<sup>1/</sup> USGS records are available from June 1939 through 1956. Runoff for 1925 through May 1939 was estimated by correlation with Ashley Creek near Vernal minus diversions to Ashley Creek from Oaks Park Reservoir on Brush Creek.

<sup>2/</sup> 1941-1956 is the period during which additional water was diverted to Ashley Creek from Oaks Park Reservoir on Brush Creek.

<sup>3/</sup> 1925-1956 is the period of study.



the regulation of upstream storage at the glacial lakes and Long Park Reservoir is less than one percent of the Ashley Creek runoff, no adjustments were made for its effect. This is justified because the correction would not be within the accuracy of stream flow measurements.

#### Ashley Creek near Vernal

The station is located  $4\frac{1}{2}$  miles upstream from Dry Fork, a tributary to Ashley Creek, and  $5\frac{1}{4}$  miles upstream from the station Ashley Creek at "Sign of the Maine." Discharge records are available from October 1911 through September 1956. This station has been operated almost continuously since its installation. However, it has been operated at three different locations, and the final location established in October 1914 is the only record of value in the present studies. The present location near the Utah Power and Light Company's hydroelectric plant is below the Ashley Creek Spring and above Dry Fork, the main tributary of Ashley Creek. The record, therefore, provides only data on the main fork of Ashley Creek and does not give information on the total available supply for storage in the Stanaker Reservoir. However, this station does serve for comparison with the later established station, at "Sign of the Maine," below Dry Fork, in developing a record of water available for unit use. This station is above all diversions for irrigation uses and is used as the basic long time record station. Elevation of the station is 6,250 feet and the drainage area covers 101 square miles. The natural flow at this gage is increased by water released from Oaks Park Reservoir through the Oaks Park Canal. Municipal water for the city of Vernal and surrounding area is diverted from a tributary spring about 1,000 feet above the station. Table No. 5 gives the monthly summary of the natural flow at "Ashley Creek near Vernal" station for a period of 1925 through 1956.

#### Oaks Park Canal near Vernal

The station is located 1,000 feet downstream from Oaks Park Dam and 200 feet downstream from the point of diversion from Brush Creek. This record gives the transmountain diversion from Brush Creek drainage to the Ashley Creek drainage. Surplus Brush Creek water not required by downstream users in the Brush Creek drainage is stored in Oaks Park Reservoir, active capacity 5,740 acre-feet, and released as needed to Ashley Creek through the Oaks Park Canal. Discharge records at this station are available from April 1946 to September 1956. The divertible supply from Brush Creek is limited by the storable flow at the reservoir and the reservoir capacity. Table No. 6 gives the monthly summary of flows diverted through the Oaks Park Canal.

#### Oaks Park Reservoir

Data on stream flow at the Oaks Park Reservoir are limited. Table No. 37 gives the summary of the storable flow of Brush Creek at Oaks Park

Table No. 5

Flow of Ashley Creek near Vernal minus Oaks Park Canal Diversion <sup>1/</sup>

Unit: 1000 a.f.

Year	Oct.	Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Total
1925	2.4	2.1	2.0	1.9	1.7	1.8	3.4	14.2	11.8	6.8	5.0	5.4	58.5
26	6.7	4.6	3.2	2.4	1.8	1.9	8.4	20.9	9.0	6.0	4.8	3.0	72.7
27	2.7	2.1	2.0	1.7	1.5	1.6	2.5	26.1	16.9	9.1	6.4	13.7	86.3
28	7.0	5.4	3.4	2.8	2.3	2.1	4.2	33.8	12.8	6.4	4.6	2.8	87.6
29	3.1	2.8	2.1	2.3	1.8	1.8	2.5	27.3	31.4	9.3	6.5	6.3	97.2
1930	5.6	4.1	3.1	2.8	2.0	2.1	6.5	23.9	15.2	6.3	7.0	5.6	84.2
31	5.5	3.4	2.8	2.5	1.9	2.0	2.7	4.3	5.5	2.8	2.3	1.9	42.6
32	2.0	1.9	1.8	1.8	1.7	1.7	2.0	24.5	18.3	8.3	5.6	4.4	74.0
33	3.6	2.7	1.7	1.5	1.3	1.3	1.3	9.3	15.4	4.6	3.6	2.2	48.5
34	1.8	1.5	1.3	1.3	1.1	1.2	3.8	7.8	3.5	2.5	3.0	2.5	31.3
1935	2.1	2.0	1.8	1.6	1.4	1.6	1.7	11.8	27.1	6.6	4.0	2.3	64.0
36	1.9	1.6	1.4	1.2	1.1	1.1	2.2	11.4	6.7	4.3	4.8	4.0	41.7
37	3.3	2.5	1.6	1.4	1.0	1.1	2.3	31.5	15.6	8.7	5.7	3.9	78.6
38	3.0	2.1	1.6	1.4	1.2	1.3	2.4	20.3	24.5	8.9	5.5	4.8	77.0
39	6.7	4.8	3.3	2.5	1.9	2.1	7.7	17.7	7.2	4.3	2.9	5.3	66.4
1940	5.6	3.5	2.2	1.6	1.3	1.4	4.6	17.5	6.9	4.1	2.2	3.0	53.9
41	4.8	3.3	2.3	1.9	1.6	1.7	1.6	30.2	22.3	7.9	5.7	6.0	89.3
42	9.4	6.2	4.0	2.6	1.8	1.8	6.0	22.4	25.3	8.4	5.0	2.7	95.6
43	2.6	1.9	1.6	1.3	1.0	1.1	8.8	16.6	10.5	5.7	3.8	2.1	57.0
44	2.2	1.7	1.5	1.3	1.1	1.1	1.2	23.0	36.1	10.8	5.1	2.8	87.9
1945	2.5	2.0	1.6	1.4	1.1	1.2	1.3	15.4	15.4	7.0	5.2	3.8	57.9
46	3.2	2.2	1.6	1.4	1.1	1.1	6.4	9.5	6.1	3.3	3.1	3.6	42.6
47	3.4	2.9	2.4	1.9	1.5	1.6	3.8	31.5	20.0	9.4	5.8	4.3	88.5
48	3.1	2.6	2.1	1.7	1.3	1.3	1.9	25.0	12.6	5.1	2.8	1.6	61.1
49	1.6	1.4	1.2	1.0	.9	.9	2.5	27.2	23.6	7.5	4.7	2.7	75.2
1950	2.9	2.4	1.8	1.7	1.2	1.4	5.6	25.1	22.5	9.6	4.8	3.7	82.7
51	2.5	2.0	1.8	1.6	1.2	1.2	1.2	15.9	12.1	6.0	5.4	4.1	55.0
52	3.3	2.3	1.9	1.8	1.5	1.4	4.2	36.3	23.2	8.5	9.4	5.1	98.9
53	3.0	2.3	2.1	1.8	1.4	1.4	1.8	9.5	17.2	5.6	2.8	1.8	50.7
54	1.9	1.7	1.7	1.6	1.1	1.1	3.9	18.8	7.1	4.8	3.2	2.3	49.2
1955	2.4	1.8	1.5	1.3	1.0	1.1	1.3	15.9	10.3	4.3	3.7	2.6	47.2
56	2.1	1.7	1.5	1.4	1.2	1.2	2.1	23.5	10.5	5.0	2.3	1.4	53.9
1925-56													
Total	113.9	85.5	65.9	56.4	45.0	46.7	111.8	653.1	502.6	207.9	146.7	121.7	2,157.2
Mean	3.6	2.7	2.0	1.8	1.3	1.5	3.5	20.4	15.7	6.5	4.6	3.8	67.4

<sup>1/</sup> USGS records. Oaks Park Canal diverts from Brush Creek to Ashley Creek above this gage.



Table No. 6

Flow of Oaks Park Canal near Vernal 1/

Unit: 1000 a.f.

Year	Oct.	Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Total
1941										1.4	.9	.8	3.1
42										1.1	2.0	2.3	5.4
43	.6									2.6	2.0	1.1	6.3
44										1.3	3.2	1.4	5.9
1945	.1									1.5	1.5	1.5	4.6
46	.6	.1						.1	1.9	1.8	.2	.2	4.9
47	0							.1	.1	1.2	1.2	1.1	3.7
48	1.3								.2	2.0	2.4	.8	6.7
49	.2								.2	1.3	2.5	1.0	5.2
1950	.3									.6	2.6	1.2	4.7
51	.8									2.2	1.1	.6	4.7
52	.6									1.9	.5	.7	3.7
53	1.3	.4								2.5	2.7	.5	7.4
54									1.0	2.2	.7	.3	4.2
1955									.2	1.9	.2	.1	2.2
56										2.3	1.5	.6	4.4
Total	5.8	.5						.2	3.6	27.8	25.2	14.2	77.3
Mean	.4	0						0	.2	1.7	1.6	.9	4.8

1/ May 1946 - 1953 USGS Records  
 July 1941 - 1946 Ashley Irrig. Co. Records

Reservoir from 1925 to 1955. Storable flow for 1942 to 1946 is based upon records of outflow and reservoir gage heights. Data for other years was estimated by correlation with "Ashley Creek near Vernal" minus Oaks Park Canal.

#### Ashley Creek near Jensen

The station is located immediately downstream from the U.S. Highway 40 bridge, 3 miles upstream from the junction of Ashley Creek and the Green River, 3 miles west of Jensen, Utah. Discharge records at this station are available from October 1946 to September 1956. Elevation of the station is 4795 feet and the drainage area is 386 square miles. The gage is below all stream diversions except for the Union Canal which diverts from Ashley Creek about 2,000 feet downstream from the gage. Table No. 7 gives the monthly summary of the flows of Ashley Creek near Jensen.

#### Available Supply

The water supply for Ashley Valley is presently obtained from Ashley Creek which drains some 238 square miles above the valley. Runoff due to the nature of the water shed is rapid, resulting in wide fluctuations in stream flow as shown by the water records at the "Sign of the Maine" gage in Table No. 3. Winter and late summer flows are low compared to the spring flows. Average monthly flow for September through April is 2,600 acre-feet while the average for May is 29,000, June 27,800, July 9,800, and August 6,300 acre-feet for the period 1941 to 1956.

#### Past Diversions to Canals

By court decree all of the waters of Ashley Creek were divided among the Stanaker, Ashley Upper, Ashley Central, Colton, Hardy, Island, Dodds, and Rock Point Canals. Since that time and to the present these canals have diverted all of the flows of Ashley Creek up to the capacity of their respective canals and in the proportion granted under the decree. The Highline Canal was not included in the decree but has since acquired a high water filing on Ashley Creek. Under present operating conditions the Stanaker, Ashley Upper, Ashley Central, Dodds, Island, and Rock Point Canals divert all the water in Ashley Creek at the "Sign of the Maine" gage up to 500 cfs, with the Highline diverting such flows between 500 cfs and 682 cfs. The excess flows are diverted by the lower canals, River and Union, or spills into the Green River below Jensen.

Recorded data on the past diversions to the various canals is limited to fragmentary records developed from staff gage readings. The general practice is to divert water from Ashley Creek throughout the year. Irrigation begins in the latter part of March or early April and continues into November. Nonirrigation season flows are used for stock watering



Table No. 7

## Runoff of Ashley Creek near Jensen\*

Unit: 1000 a.f.

Year.	Oct.	Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Total
1942													
43	1.4					1.7	2.5	5.5	6.0	.5	.5	.7	
44	.8												
1945													
46													
47	.4	1.2	2.2	3.4	3.2	3.0	1.1	31.3	30.1	2.2	1.6	.9	80.6
48	1.8	2.8	3.3	2.9	3.1	4.0	2.1	12.4	5.4	.5	.3	.2	38.8
49	.5	1.0	1.5	2.0	2.3	4.0	1.3	11.4	30.9	2.0	.5	.6	58.0
1950	3.8	3.8	3.4	3.4	2.8	5.5	6.2	23.6	19.8	4.3	.9	1.3	78.8
51	2.2	2.6	3.2	2.8	2.5	2.0	.7	7.7	5.5	.5	.7	1.1	31.5
52	3.6	3.3	3.3	3.0	2.6	4.1	9.9	31.7	24.4	1.9	3.1	2.3	93.2
53	2.3	3.0	4.0	3.8	3.1	3.6	1.5	1.2	4.6	.2	.2	.2	27.7
54	.5	1.4	2.4	2.8	2.2	1.6	.7	2.6	1.3	.2	.1	.2	16.0
1955	1.0	1.2	2.1	2.2	1.9	3.1	1.3	.9	1.6	.1	.1	.1	15.6
1947-55													
Total	16.1	20.3	25.4	26.3	23.7	30.9	24.8	122.8	123.6	11.9	7.5	6.9	440.2
Mean	1.8	2.3	2.8	2.9	2.6	3.4	2.8	13.7	13.7	1.3	.8	.8	48.9

\* USGS Records - 1947-1954

USBR Records - 1942-1944

Table No. 8  
Summary of Diversions to Canals 1951-1955

Unit: A.F.

Year	Month	Ashley Central		Island Total	Rock Point		Dodds Total	Upper Ashley		Highline		Stanaker		Total Res.	Total in Canals
		Res.	Total		Res.	Total		Res.	Total	Res.	Total	Res.	Total		
1951	May	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	June	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	July	222	2011	457	77	1229	65	956	2948	851	990	12	151	2118	7851
	Total	222	2011	457	77	1229	65	956	2948	851	990	12	151	2118	7851
1952	July	250	2380	529	-	1458	105	853	2666	960	111	-	193	2063	7442
	Aug.	113	3197	712	-	944	99	383	3911	522	336	-	157	1018	9356
	Sept.	133	1819	417	-	988	61	448	1880	75	97	12	111	668	5373
	Oct.	177	1137	250	-	659	54	643	1305	36	54	14	69	870	3528
	Total	673	8533	1908	-	4049	319	2327	9762	1593	598	26	530	4619	25699
1953	July	345	1808	358	228	982	56	1192	2901	760	785	-	95	2549	7125
	Aug.	305	1568	274	313	1017	42	1176	2604	367	411	36	105	2197	6012
	Sept.	105	744	159	4	411	21	248	1029	38	14	-	36	395	2414
	Total	775	4180	791	545	2410	119	2616	6614	1165	1210	40	236	5141	15559
1954	May	-	536	754	-	1967	113	-	9261	-	-	-	284	-	17746
	June	117	2636	6758	-	1589	96	692	3780	553	172	-	193	1362	15224
	July	407	2102	3901	30	1067	60	934	2912	597	80	8	117	1976	10239
	Aug.	454	1464	219	44	619	32	391	1428	42	44	4	67	935	3873
	Sept.	79	714	135	26	406	16	89	908	-	16	4	36	198	2231
	Total	1057	12283	8767	100	5648	317	2106	18289	1192	312	16	697	4471	46313
1955	June	22	3689	781	-	1663	105	89	3578	516	-	-	210	627	10026
	July	430	1779	315	63	877	41	1000	2543	200	204	-	82	1693	5841
	Aug.	160	1289	238	46	650	27	274	1521	28	12	14	56	522	3793
	Sept.	-	810	202	-	529	26	10	1123	-	-	-	59	10	2809
	Oct.	-	422	95	-	573	8	167	573	-	-	-	57	167	1728
	Total	612	8049	1631	109	4292	207	1540	9338	744	216	14	464	3019	24197

Table 9

## Monthly Discharge of Canals in Ashley Valley

Stream	Year	Mar.	April	May	June	July	Aug.	Sept.	Oct.	Total
High Line	1943	93	840	1,830	3,380	634	22	48		6,847
Canal	1944			2,405	5,900	3,820	625	304	60	13,114
Ashley	1942						2,840	2,300	1,950	7,090
Upper	1943	560	4,000	10,500	7,702	3,410	2,430	1,310		29,912
Canal	1944			7,410	12,290	6,540	3,370	1,500	1,142	32,252
Ashley	1942						2,030	1,820	1,040	4,890
Central	1943	415	1,930	5,870	3,920	2,380	1,921	960		17,393
Canal	1944		298	4,560	7,180	3,980	2,390	1,340	970	20,718
Rock	1942						330	603	350	1,783
Point	1943	256	792	2,118	1,040	980	935	540		6,661
Canal	1944		72	644	1,542	2,312	1,096	782	388	6,836
Island	1942						308	243	247	798
Ditch	1943	34	404	800	694	395	270	153		2,752
	1944			791	884	887	381	211	162	3,316
River-	1942						194	122	41	357
dale	1943	0	284	630	358	164	103	71		1,610
Canal	1944									

purposes, or wasted. No attempt is made to control the use of water so long as the flow is above 500 cfs; consequently, it is common practice to apply more water than can be beneficially used. As the flow of water in the creek diminishes closer control is placed over diversions. Each canal having a right to use of water is entitled to a certain percentage of the total flow, and the practice is to apportion the limited supply as equitably as possible with limited measuring devices under the supervision of the River Commissioner. Available records of irrigation diversions during the periods 1942-44 and 1950-55 compared with estimated requirements reflect the practice of diverting excessive amounts of water during the high runoff period. These records are summarized in Table Nos. 8 and 9. Following the high runoff season (April to June) the natural flow of Ashley Creek diminishes rapidly, leaving the area with insufficient water for late season crops.

Supplemental water is provided by storage Oaks Park Reservoir, Long Park Reservoir, and a group of small glacial lakes. Total storage of these reservoirs is estimated to be 6,870 acre-feet. Oaks Park Reservoir with an active capacity of 5,740 acre-feet, stores the surplus spring flows of Brush Creek. This storage is released in the late summer months as given in Table No. 6. The reservoir capacity is divided among the various canals and municipal water uses in accordance with shares of Ashley Valley Reservoir Company stock owned by the water users. Table No. 10 gives the summary of Oaks Park Reservoir ownership for 1956.

Table 10  
Ashley Valley Reservoir Company  
Distribution of Stock by Canals  
(for 1956)

<u>Canal</u>	<u>Shares of Stock</u>
Stanaker	108.00
Highline	4,407.46
Ashley Upper	9,991.50
Ashley Central	5,235.52
Rock Point	1,165.76
Island	20.00
City Pipeline	1,564.55
Total	22,492.79

Long Park Reservoir located on the headwaters of the North Fork of Ashley Creek has an estimated capacity of 520 acre-feet. An estimated 610 acre-feet of storage is contained in small glacial lakes in the headwaters of the South Fork of Ashley Creek.

Even with this supplemental supply the natural flow of Ashley Creek and storage releases are insufficient to supply adequately the needs of presently irrigated lands.



## Municipal water system

Municipal water for Vernal, Maeser, and Naples areas is diverted from Ashley Creek springs on Ashley Creek above all irrigation diversions, and about 9 miles above Vernal. The spring area has been enclosed in a large masonry box with an overflow section. The Geological Survey has kept a record of flows in the overflow channel below the spring for 1944 and 1945, and since August 1954. The records indicate a minimum flow of about 15 second-feet and a maximum of about 49 second-feet, in addition to the flow in the pipeline. The flows are shown as follows:

Unit: Acre-feet													
Year	Oct.	Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sep.	Total
1944	1490	1300	1180	1010	883	928	996	2330	2810	2770	2590	1930	20,220
1945	1690	1490	1280	1180	1000	1100	1140	2440	2550	2570	2390	2180	21,060
1954											1830	1600	--
1955	1600	1440	1330	1160	964	1010	1140	2480	2450	2240	1970	1670	19,450

A 12-inch steel pipeline conveys the water from the masonry box to a chlorination plant near the head of Ashley Valley. The capacity of the pipeline is about 7 second-feet. Under present operations the flow of the pipeline is varied from about 3 second-feet in the winter to about 5 second-feet in summer. After leaving the chlorination plant, which is operated only when there is evidence of pollution, the water is measured through a weir into a division box from which about 1/8 of the flow goes to the Maeser area. The remainder is piped to the Vernal City system, which consists of a 75,000 gallon concrete reservoir, a 500,000 gallon concrete reservoir, and the distribution pipelines. Excess water is spilled from the reservoir into Ashley Central Canal. The Naples system connects with the Vernal City pipeline near the southwest corner of Vernal. Records of flow at the measuring weir from May 1952, to September 1956, are summarized in Table 11.

Records of past water use were obtained for Vernal City, Maeser, and Naples for parts of the years 1953, 1954, and 1955, from a survey of meter readings as obtained from city officials. Actual meter readings were increased by 33 1/3% to account for leakage and unmetered connections and are summarized in Table 12.

Estimated population as served by the pipeline in 1956 is 4300 in Vernal, 912 in Maeser, and 608 in Naples. The average use for 1955 amounted to 224 gallons per person per day.

Rights to the use of Ashley Creek water are based upon the court decree of 1897, in which the entire flow of the stream was divided among the water users. In effect, each water user is entitled to a percentage



Table 11

Flow of Municipal Pipeline <sup>1/</sup>  
for Vernal City, Maeser, and Naples  
at Chlorination Plant

Unit: Acre-feet													Total
Year	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Oct.	Nov.	Dec.	
1952					240	260	271	271	262	269	245	182	
1953	185	175	191	205	291	282	293	293	283	193	179	185	2,755
1954	185	167	185	245	295	298	307	306	244	290	178	173	2,873
1955	173	156	173	191	271	278	307	307	298	289	195	179	2,817
1956	179	167	188	236	224	249	307	307	297				

<sup>1/</sup> Measured over a weir at which a staff gage is read periodically.  
These figures are therefore only approximate.

Table 12

Past Municipal Water Use  
Vernal City, Maeser, and Naples

Unit: Acre-feet												
Year	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Oct.	Nov.	Dec. Total
<b>Vernal City</b>												
1953	38	37	54	75	91	147	156	123	108	66	49	53 --
1954	53	48	55	102	158	136	155	138	84	54	46	59 --
1955	59	54	57	98	137	164	165	133	127	93	53	53 1193
<b>Maeser Area</b>												
1954							22	20	12	11	9	9 --
1955	9	8	8	13	20	23	19	15	15	17	9	9 165
<b>Naples Area</b>												
1953												4 --
1954	4	4	6	9	10	7	9	7	9	6	5	6 --
1955	5	5	6	9	12	12	12	10	12	8	5	4 100
<b>Total System</b>												
1954							186	165	105	71	61	74 --
1955	73	67	71	120	169	199	196	157	153	119	68	67 1459

## WATER SUPPLY

## WATER RESOURCES

of the total flow as measured at "Sign of the Maine" gage. Municipal water rights have been obtained by purchasing stock in the Ashley Upper and Ashley Central canal companies, plus the transfer to Vernal City of the A. J. Johnson right as set forth in the court decree. Municipal water rights are entitled to 4.34% of Ashley Creek flows as follows:

Item	Percent of Ashley Creek Flow			
	Vernal	Maeser	Naples	Total
Ashley Upper Irrigation Co.	0.39	0.18	-	0.57
Ashley Central Irrigation Co.	2.60	0.38	0.46	3.44
A. J. Johnson Right	0.33	-	-	0.33
Total Direct Flow	3.32	0.56	0.46	4.34

In addition to rights on Ashley Creek, the municipal water system owns stock in the Ashley Valley Reservoir Company, which has storage of 5,740 acre-feet in Oaks Park Reservoir, 520 acre-feet in Long Park Reservoir, 360 acre-feet in Twin Lakes, 150 acre-feet in Goose Lake, and 100 acre-feet in Mirror Lake, making a total of 6,870 acre-feet. In 1956 the company had about 22,493 shares of stock, of which the municipal water system owned about 1,565 shares. This is about 7% of the stock, or 480 acre-feet of storage.

Ashley Creek spring is an excellent source of municipal water, and physically, there is an abundance of water available for future growth of the municipal system. With respect to water rights, however, the municipal system owns only enough water for present requirements. If a year of runoff like 1934 were to occur, the Ashley Creek flow owned by the municipal system would fail to meet the demand similar to that of 1955 by 380 acre-feet. This shortage could possibly be made up from storage in the upstream reservoirs, provided the storage space filled during the spring runoff. It is very doubtful if runoff would be sufficient in a year like 1934 to fill the reservoir more than about 1/3 full.

### Quality of Water

Quality of water may be defined as the suitability of water for irrigation use. It is measured by the type and concentration of the dissolved and suspended constituents. Experience and research indicate that a minimum of three factors must be known in order to make an estimate of water quality for irrigation use: (1) The total concentration of dissolved solids, (2) the relative proportion of sodium to other cations, and (3) the concentration of boron.

A relatively simple method of classifying irrigation waters with regard to the first two factors has been developed by the Agriculture Research Service of the Department of Agriculture. With respect to

total dissolved solids, as measured by conductivity, waters are divided into four classes. The dividing points between classes are at 250, 750, and 2,250 micromhos/cm. The classes are described as follows:

C1 Low-salinity water can be used for irrigation with most crops, on most soils with little likelihood that soil salinity will develop. Some leaching is required, but this occurs under normal irrigation practices, except in soils of extremely low permeability.

C2 Medium-salinity water can be used if a moderate amount of leaching occurs. Plants with moderate salt tolerance can be grown in most cases without special practices for salinity control.

C3 High-salinity water cannot be used on soils with restricted drainage. Even with adequate drainage, special management for salinity control may be required, and plants with good salt tolerance should be selected.

C4 Very High-salinity water is not suitable for irrigation under ordinary conditions, but may be used occasionally under very special circumstances. The soils must be permeable, drainage must be adequate, irrigation water must be applied in excess to provide considerable leaching and very salt-tolerant crops should be selected.

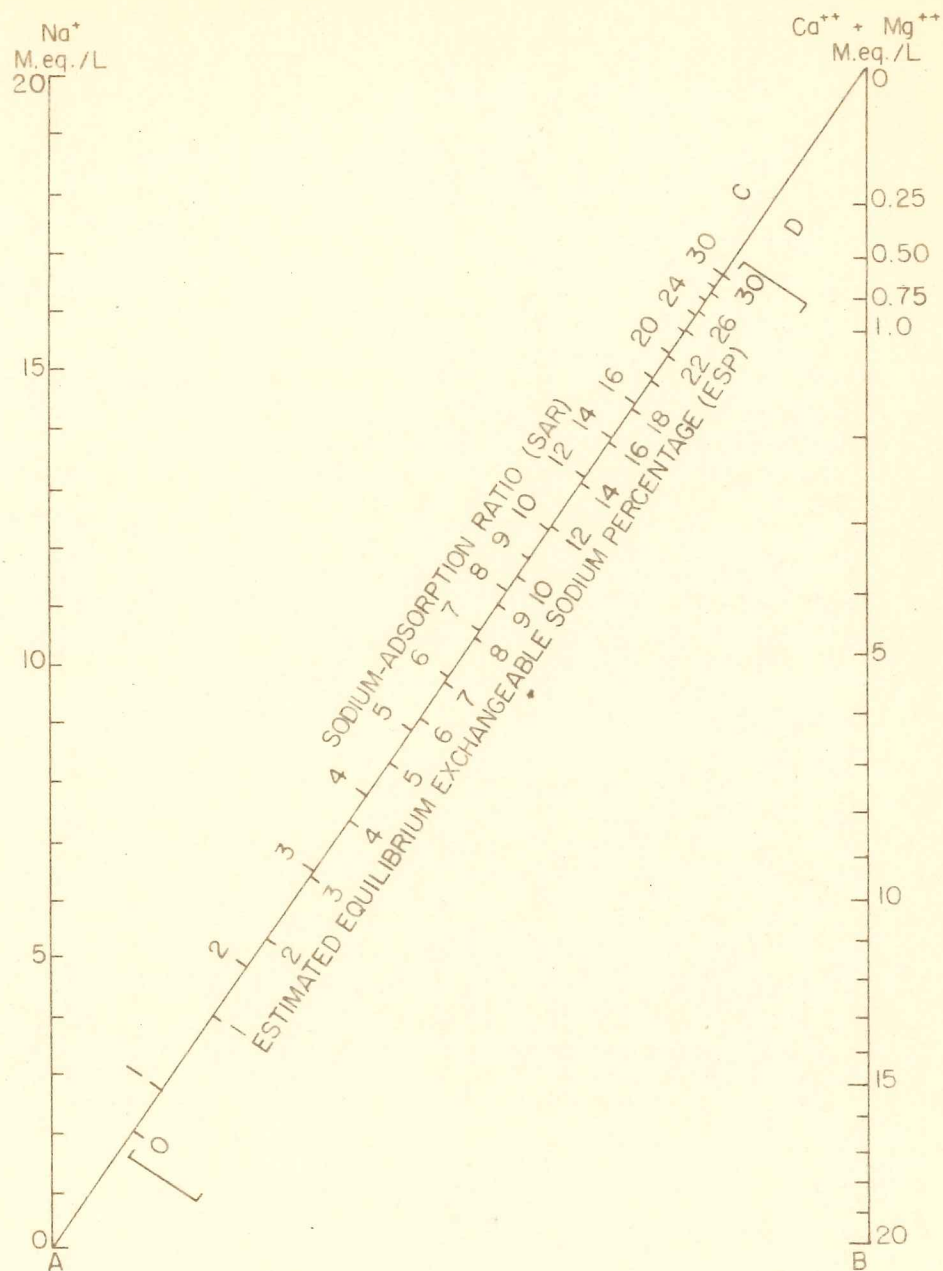
The relative proportion of sodium to other cations in an irrigation water is expressed in terms of "sodium-adsorption-ratio" (SAR) which is defined by the equation:

$$SAR = \frac{Na^+}{\frac{Ca^{++} + Mg^{++}}{2}}$$

where  $Na^+$ ,  $Ca^{++}$ , and  $Mg^{++}$  represent the concentrations in milliequivalents per liter (m.e./l.) of sodium, calcium, and magnesium, respectively. The sodium-adsorption-ratio is more significant for interpreting water quality than the "percent sodium" formerly used because it relates more directly to the adsorption of sodium by the soil. A nomogram for estimating the SAR value of an irrigation water is shown in Figure 1. The SAR value may be calculated from the above equation or estimated by use of the nomogram. A very preliminary estimate of the exchangeable sodium percentage which may result in a well-drained soil at equilibrium with the irrigation water may be read on the D scale. Waters are divided into four classes, with respect to sodium hazard, the dividing points being at SAR values of 10, 18, and 26, see Figure 2. The classes are described as follows:

S1 Low-sodium water can be used for irrigation on almost all soils with little danger of the development of harmful levels of exchangeable sodium. However, sodium-sensitive crops such as stonefruit

# NOMOGRAM FOR ESTIMATING SODIUM ADSORPTION RATIO AND EXCHANGEABLE SODIUM PERCENTAGE\*

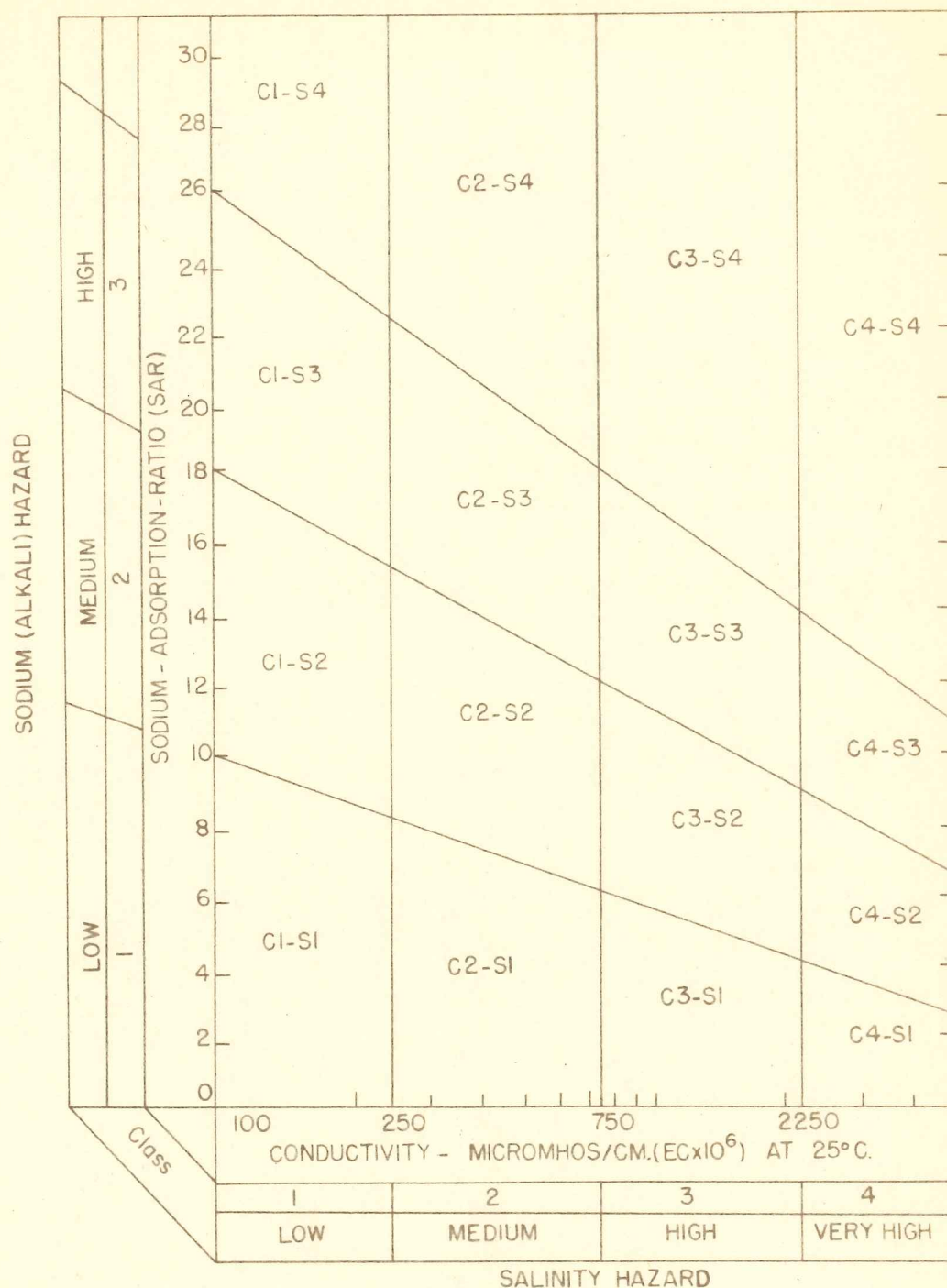


\* Source: "The Diagnosis and Improvement of Saline and Alkali Soils."  
U.S. Dept. Agr. Handbook (in process), 1953.

FIGURE 1



DIAGRAM FOR USE IN INTERPRETING THE ANALYSIS OF  
IRRIGATION WATER\*



\* Source: "The Diagnosis and Improvement of Saline and Alkali Soils."  
U.S. Dept. Agr. Handbook (in process), 1953

trees and avacados may accumulate injurious concentrations of sodium.

S2 Medium-sodium water will present an appreciable sodium hazard in fine-textured soils having high cation-exchange-capacity, especially under low leaching conditions unless gypsum is present in the soil. This water may be used on coarse-textured or organic soils with good permeability.

S3 High-sodium water may produce harmful levels of exchangeable sodium in most soils and will require special soil management: good drainage, high leaching, and organic matter additions. Gypsiferous soils may not develop harmful levels of exchangeable sodium from such waters. Chemical amendments may be required for replacement of exchangeable sodium, except that amendments may not be feasible in the case of waters of very high salinity.

S4 Very High-sodium water is generally unsatisfactory for irrigation purposes, except at low and perhaps medium salinity where the solution of calcium from the soil or use of gypsum or other amendments may make the use of these waters feasible.

Boron is present in practically all waters. It is essential to plant growth, but is exceedingly toxic at concentrations only slightly above optimum. The permissible limits for boron vary largely with crop sensitivity and are summarized in the following table:

Table of boron content  
permissible limits for irrigation water<sup>1/</sup>

Classes of Water		Boron, parts per million		
Rating	Grade	Sensitive crops	Semitolerant crops	Tolerant crops
1	Excellent	0.33 or less	0.67 or less	1.00 or less
2	Good	0.33 to 0.67	0.67 to 1.33	1.00 to 2.00
3	Permissible	0.67 to 1.00	1.33 to 2.00	2.00 to 3.00
4	Doubtful	1.00 to 1.25	2.00 to 2.50	3.00 to 3.75
5	Unsuitable	1.25 or more	2.50 or more	3.75 or more

<sup>1/</sup> Wilcox, L. V., "The Quality of Water for Irrigation Use,"  
U. S. Department of Agriculture, Technical Bulletin 962, O. 40, 1948.

Crops have been grouped in accordance to their tolerance to boron as shown in Table 13. Most of the crops expected to be grown in the Vernal area fall within the Semitolerant group, so the ratings of this group have been applied to waters of Vernal Unit.



Table 13Relative Tolerance of Crop Plants to Boron 1/

Sensitive	Semitolerant	Tolerant
Lemon	Lima bean	Carrot
Grapefruit	Sweet potato	Lettuce
Avocado	Bell pepper	Cabbage
Orange	Tomato	Turnip
Thornless blackberry	Pumpkin	Onion
Apricot	Zinniz	Broadbean
Peach	Oat	Gladiolus
Cherry	Milo	Alfalfa
Persimmon	Corn	Garden beet
Kadota fig	Wheat	Mangel
Grape (Sultanina and Malaga)	Barley	Sugar beet
Apple	Olive	Palm (Phoenix canariensis)
Pear	Ragged Robin rose	Date palm (P. dactylifera)
Plum	Field pea	Asparagus
American elm	Radish	Athel (Tamarix aphylla)
Navy bean	Sweet pea	
Jerusalem-artichoke	Pima cotton	
Persian (English) walnut	Acala cotton	
Black walnut	Potato	
Pecan	Sunflower (native)	

- 1/ Eaton, F. M., "Boron in Soils and Irrigation Waters and its Effects on Plants, with Particular Reference to the San Joaquin Valley of California". U. S. Department of Agriculture, Technical Bulletin 448, p. 60, 1941.

### Sampling pattern

A network of water sampling stations was established in order to determine the quality of water at points on Ashley Creek and various canals and drains throughout Ashley Valley. Drawing No. 325-418-66 gives the location and number of each station.

### Analysis

Experience has shown that the water is of good quality when it enters Ashley Valley, but its quality decreases progressively as it flows through the valley and picks up return flows from irrigation. This is illustrated in Table 14.

Return flow to Ashley Creek is generally of poor quality and water entering Ashley Creek from both sides of the valley has a rather high salinity hazard. Table 15 summarizes the quality of water entering Ashley Creek from drains and seeps on each side of the stream.

Two areas on the west side of Ashley Creek and one on the east side are producing water which is exceptionally high in salinity hazard. These are the South Fork of Naples Drain, a seep area near Ashley Creek upstream from Naples Drain, and a spring on the east side of Ashley Creek just south of the Diamond Mountain road. Water from the spring has reportedly caused sickness among cattle grazing in the area. Table 16 summarizes results of sampling of these stations. North Fork of Naples Drain is included in the table for comparison with the South Fork.

Laboratory analysis of all samples taken at each of the key sampling stations are given in Table 17.

From results of the sampling program it is evident that the total concentration of salts presents a much greater hazard than either the sodium or boron concentrations. It is also evident that the higher concentrations of salts occur during the latter part of the irrigation season when stream flows are lowest and when return flow constitutes the largest proportion of the stream flow of any time during the year.

The Union and River canals are located near the lower end of Ashley Valley. At the present time they are receiving a poor quality of water, especially during the late part of the irrigation season, as then supply consists almost entirely of return flow except for a short period during the high water season.

Construction of the Vernal Unit is not anticipated to cause any material change in the quality of return flow water, but due to the present poor quality, the sampling program will be continued in order that sufficient factual data will be available for evaluating the quality conditions with and without the Vernal Unit.

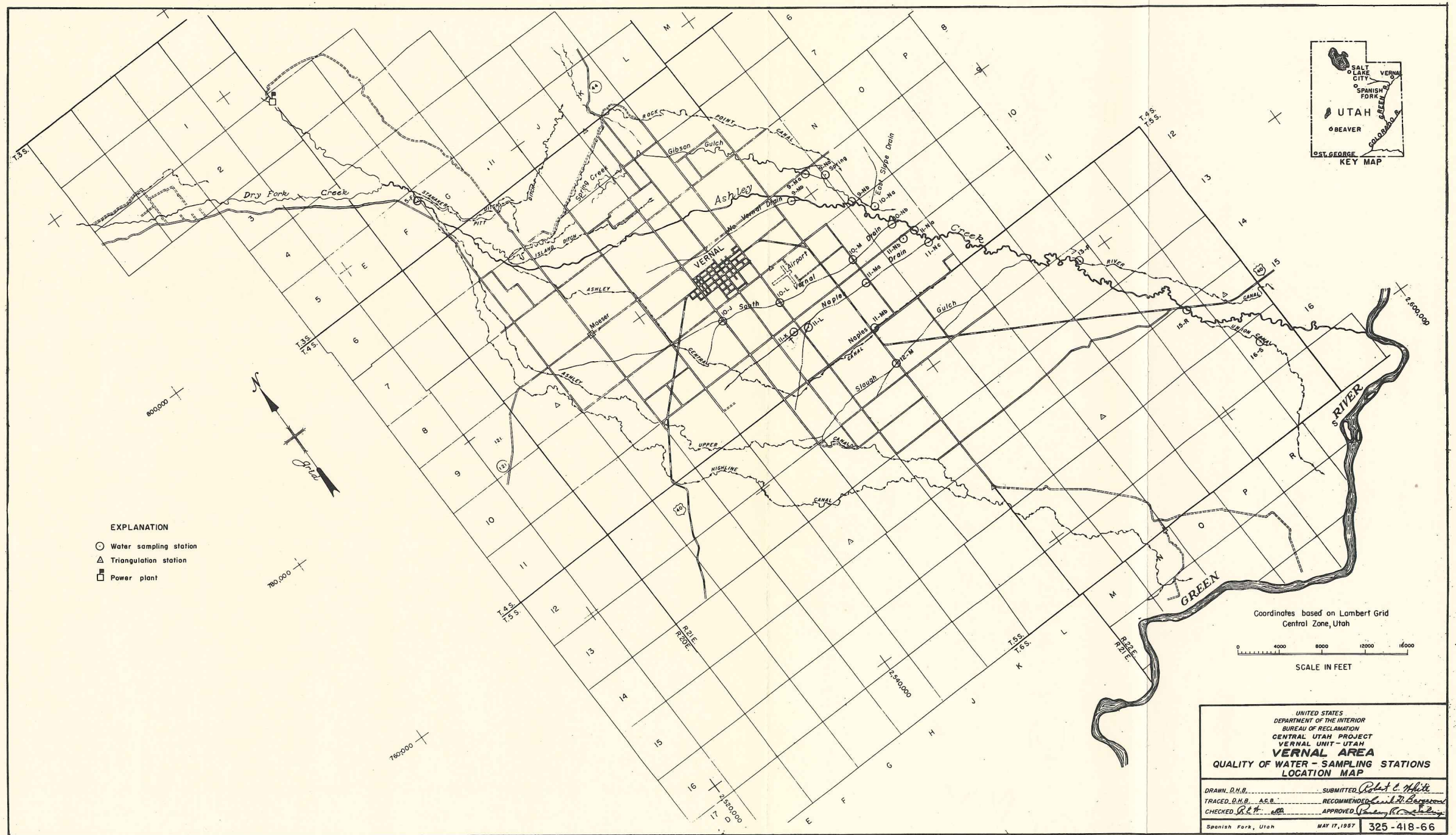




Table 14

Quality of Water in Ashley Valley  
 Illustrating Decrease in Quality as Flow Proceeds Downstream

Date	5-F				11-Mb				11-Na				13-P				15-R				16-S			
	Ashley Creek at Sign of the Maine				Ashley Central Canal at Naples				Ashley Creek above Naples Drain				Ashley Creek at River Canal				Ashley Creek near Jensen				Union Canal at USBR Gage			
	Class				Class				Class				Class				Class				Class			
	Flow	C	S	B	Flow	C	S	B	Flow	C	S	B	Flow	C	S	B	Flow	C	S	B	Flow	C	S	B
7/21/55	103	1	1	1					-	3	1	1					-	4	2	3				
9/26-27/55	-	1	1	1					-	3	1	1	-	4	2	1	-	4	2	1				
3/13-14/56	27	2	1	1	5	3	1	1									29.3	3	1	1				
5/24-25/56	797	1	1	1	10	2	1	1				-	2	1	1									
7/9-10/56	125	1	1	1	9	3	1	1				4	4	1	1	8.47	4	1	2	4	4	1	2	
8/ 7/56	59	1	1	1	7	2	1	1								.98	4	2	2					
9/17/56	28	1	1	1																				

C = Total concentration of salts.

S = Relative proportion of sodium to other cations.

B = Boron concentration.

Class 1 = Excellent to good, suitable for most plants under most conditions.

Class 2 = Good to injurious, probably harmful to more sensitive crops.

Class 3 = Injurious to unsatisfactory, probably harmful to most crops.

Class 4 = Unsuitable, probably harmful to all but the most tolerant crops.

Table 15

Quality of Water  
Drains and Seeps Entering Ashley Creek  
in Ashley Valley

Date	West Side					East Side				
	9-Mb	10-Nb	11-Nb	11-Nc	12-M	9-Ma	9-Nb	10-Na		
	North	South	Seep Area		Slaugh	Gibson		Gib. Gulch		
	Vernal	Vernal	South-	Naples	Gulch 3 mi	Gulch at	Gibson	Ditch and		
	Drain	Drain	East	Drain	above	Diamond	Gulch	East Slope		
	at Mouth	at Mouth	of Vernal	at Mouth	Mouta	Mtn. Road	at Mouth	Drain		
	Class	Class	Class	Class	Class	Class	Class	Class		
	Flow C S B	Flow C S B	Flow C S B	Flow C S B	Flow C S B	Flow C S B	Flow C S B	Flow C S B	Flow C S B	Flow C S B
9/26-27/55	- 3 1 1	- 3 1 1	-	- 4 1 2	-					
11/7-8/55			- 4 1 2			- 4 1 1	- 4 1 1	- 4 1 1		
3/13/56	3 3 1 1				3 2 1 1					
5/25/56					6 2 1 1					
6/11/56	3 3 1 1		- 4 1 2		-	- 3 1 1	-	2 3 1 1		
7/9-10/56	3 3 1 1	6 2 1 1		5 4 1 2	4 3 1 1		3 4 1 1	2 4 1 2		
8/7/56	1.5 3 1 1	.5 3 1 1		0.25 4 1 2	1.0 2 1 1		2 4 1 2	1 4 2 2		
9/18/56	.7 3 1 1			3 4 1 2	1.5 4 1 2	1 4 1 1		.3		

C = Total concentration of salts.

S = Relative proportion of sodium to other cations.

B = Boron concentration.

Class 1 = Excellent to good, suitable for most plants under most conditions.

Class 2 = Good to injurious, probably harmful to more sensitive crops.

Class 3 = Injurious to unsatisfactory, probably harmful to most crops.

Class 4 = Unsuitable, probably harmful to all but the most tolerant crops.

Table 16

Quality of Water  
Sampling Stations with Exceptionally  
High Concentrations

Date	11-K North Fork Naples Drain				11-L South Fork Naples Drain				11-Nb Seep Area South-East of Vernal				9-Na Spring south of Diamond Mtn. Road			
	Class				Class				Class				Class			
	Flow	C	S	B	Flow	C	S	B	Flow	C	S	B	Flow	C	S	B
10/25/55													-	4	1	2
11/7-8/55	1	4	1	1	1	4	2	2	-	4	1	2	-	4	1	2
6/11/56	2.5	3	1	1	1	4	1	2	-	4	1	2				
7/10/56	3	3	1	1	1	4	2	2								
8/7/56	2	3	1	1	-	4	2	2								
9/17/56	1															

C= Total concentration of salts.

S= Relative proportion of sodium to other cations.

B= Boron concentration.

Class 1 = Excellent to good, suitable for most plants under most conditions.

Class 2 = Good to injurious, probably harmful to more sensitive crops.

Class 3 = Injurious to unsatisfactory, probably harmful to most crops.

Class 4 = Unsuitable, probably harmful to all but the most tolerant crops.



Rev 6/56

# WATER ANALYSIS

Table 17

REGIONAL LABORATORY

Vernal Unit

## Chemical analysis of water for irrigation

[illegible]

WATER ANALYSIS  
REGIONAL LABORATORY

Table 17 (t.)

Vernal Unit

Collected by \_\_\_\_\_ Date \_\_\_\_\_ Project \_\_\_\_\_

Submitted by \_\_\_\_\_ Date \_\_\_\_\_ Location \_\_\_\_\_

Analyzed by \_\_\_\_\_ Date \_\_\_\_\_

Lab No	Field No	Sampling Date	EC x 10 <sup>6</sup> @ 25°C	pH	Total dis- solved salts ppm	Boron ppm	% Sodium	Sodium Adsorp- tion Ratio	Residual Carbon- ates	Equivalents per million or milliequivalents per liter							
										Cations				Anions			
										Ca	Mg	Na	K	CO <sub>3</sub>	HCO <sub>3</sub>	Cl	SO <sub>4</sub>
Station 9-Ma - Gibson Gulch at Diamond Mountain Road																	
CW58		11/8/55	2634	7.9	2380	.37	19.6	1.9		16.28	10.87	6.66	.24	1.57	5.52	.80	26.16
CW101		6/11/56	1990	7.8	1718	.32	16.9	1.3		11.87	8.52	4.20	.22	.00	6.27	.46	18.07
CW166		9/18/56	3469	7.7	3540	.55	18.7	2.3		21.79	16.68	8.85	.11	.92	5.77	.80	39.94
Station 9-Mb - North Vernal Drain at Mouth																	
CW52		9/27/55	1167	8.1	826	.10	11.2	.6		6.72	5.70	1.58	.13	.81	6.56	.54	6.22
CW75		3/13/56	934	7.7	664	.08	11.1	.6		5.08	3.99	1.15	.11	.40	5.00	.49	4.44
CW99		6/11/56	847	7.5	593	.30	9.5	.4		4.71	4.08	.93	.08	.00	5.42	.22	4.16
CW103		7/10/56	987	8.0	707	.18	10.4	.5		5.52	4.80	1.21	.10	.00	6.66	.38	4.59
CW126		8/ 7/56	1187	8.1	868	.22	11.0	.6		6.31	5.61	1.49	.08	.00	7.44	.55	5.50
Equivalent Weights										200	122	230	391	500	610	355	180



WATER ANALYSIS  
REGIONAL LABORATORY

Table 17 (cont.)

Vernal Unit

Collected by \_\_\_\_\_ Date \_\_\_\_\_ Project \_\_\_\_\_

Submitted by \_\_\_\_\_ Date \_\_\_\_\_ Location \_\_\_\_\_

Analyzed by \_\_\_\_\_ Date \_\_\_\_\_

Lab No	Field No	Sampling Date	EC x 10 <sup>5</sup> @ 25°C	pH	Total dissolved salts ppm	Boron ppm	% Sodium	Sodium Adsorption Ratio	Residual Carbonates	Equivalents per million or milliequivalents per liter									
										Cations				Anions					
										Ca	Mg	Na	K	CO <sub>3</sub>	HCO <sub>3</sub>	Cl	SO <sub>4</sub>		
Station 9-Na - Spring South of Diamond Mountain Road and East of Ashley Creek																			
CW56		10/25/55	3512	8.0	3372	.82	21.5	2.6			20.54	16.28	10.12	.10	1.48	4.61	.86	40.09	
CW57		11/8/56	3555	7.6	3502	.78	19.8	2.0,			23.26	16.58	9.86	.10	1.26	6.75	.92	40.87	
Station 9-Nb - Gibson Gulch at Mouth																			
CW66		11/8/55	3224	7.7	3102	.50	22.8	2.0			18.39	15.51	10.08	.15	.85	6.96	.98	35.34	
CW113		7/9/56	3119	8.1	2986	.53	20.7	2.1			18.56	14.34	8.64	.24	.00	6.48	.82	34.48	
CW125		8/7/56	3756	7.9	3792	.70	20.6	2.4			23.20	18.27	10.83	.19	.06	7.23	.98	44.22	
Station 10-J - South Vernal Drain 3 1/2 Miles above Mouth																			
CW63		11/7/55	1800	7.7	1599	.25	3.5	.2			14.44	8.16	.82	.04	.57	4.61	.23	18.05	
CW92		6/11/56	808	7.7	619	.09	4.2	.2			5.92	3.32	.41	.03	.08	2.87	.03	6.70	
Station 10-L - South Vernal Drain 2 1/2 Miles above Mouth																			
CW59		11/7/55	1745	7.9	1550	.26	3.6	.2			13.61	8.06	.82	.06	.87	3.57	.28	17.83	
CW93		6/11/56	935	7.7	727	.09	4.9	.2			6.44	4.29	.55	.05	.13	3.47	.06	7.67	
Average																			
												20.3	16.2	10.0	.11	1.30	4.10	.85	40.0

WATER ANALYSIS  
REGIONAL LABORATORYTable 17 (cont.)  
Vernal Unit

Collected by \_\_\_\_\_ Date \_\_\_\_\_ Project \_\_\_\_\_

Submitted by \_\_\_\_\_ Date \_\_\_\_\_ Location \_\_\_\_\_

Analyzed by \_\_\_\_\_ Date \_\_\_\_\_

Lab No	Field No	Sampling Date	EC x 10 <sup>6</sup> @ 25°C	pH	Total dissolved solts ppm	Barium ppm	% Sodium	Sodium Adsorp- tion Ratio	Residual Carbon- ates	Equivalents per million or milliequivalents per liter							
										Cations				Anions			
										Ca	Mg	Na	K	CO <sub>3</sub>	HCO <sub>3</sub>	Cl	SO <sub>4</sub>
Station 10-M South Vernal Drain 1 Mile above Mouth																	
CW67		11/7/55	1642	7.7	1436	.29	5.6	.4		10.82	9.11	1.19	.06	.92	4.53	.33	15.40
CW76		3/13/56	1565	7.9	1350	.22	4.5	.2		10.96	8.27	.90	.04	.63	4.24	.35	14.95
Station 10-Na Gibson Gulch Ditch and East Slope Drain below Junction																	
CW62		11/8/55	3031	8.0	2914	.56	24.6	2.6		14.77	15.61	9.96	.15	.60	3.17	.88	35.84
CW100		6/11/56	1803	8.0	1522	.55	28.2	2.2		8.03	7.81	6.28	.14	.00	5.00	.40	16.86
CW114		7/9/56	3441	8.0	3388	.70	22.1	2.5		20.07	16.68	10.48	.18	.40	5.78	.78	40.45
CW121		8/7/56	3963	8.0	3968	.76	24.0	2.9		21.82	19.59	13.12	.16	.49	5.71	1.04	47.45
CW167		9/18/56	4056	7.8	4240	.75	25.8	3.2		21.09	19.80	14.25	.12	1.00	5.46	.94	47.86
Station 10-Nb South Vernal Drain at Mouth																	
CW49		9/26/55	1846	8.0	1578	.14	8.2	.1		11.18	6.35	1.37	.08	1.07	4.20	.36	13.55
CW109		7/10/56	1668	7.9	1464	.30	6.4	.4		10.53	9.57	1.37	.05	.00	5.12	.28	16.12
CW119		8/7/56	1815	8.2	1644	.40	6.9	.5		10.41	11.10	1.59	.05	.16	4.03	.22	18.74
Equivalent Weights										20.0	12.2	23.0	39.1	30.0	61.0	35.5	48.0



WATER ANALYSIS  
REGIONAL LABORATORY

Table 17 (cont.)

Vernal Unit

Collected by \_\_\_\_\_ Date \_\_\_\_\_ Project \_\_\_\_\_

Submitted by \_\_\_\_\_ Date \_\_\_\_\_ Location \_\_\_\_\_

Analyzed by \_\_\_\_\_ Date \_\_\_\_\_

Lab No	Field No	Sampling Date	ECx10 <sup>6</sup> @ 25°C	pH	Total dissolved salts ppm	Boron ppm	% Sodium	Sodium Adsorption Ratio	Residual Carbonates	Equivalents per million or milliequivalents per liter								
										Cations				Anions				
										Ca	Mg	Na	K	CO <sub>3</sub>	HCO <sub>3</sub>	Cl	SO <sub>4</sub>	
Station 11-K North Fork of Naples Drain above Confluence with South Fork																		
CW60		11/7/55	2217	7.9	1864	.33	13.2	1.1			14.20	10.08	3.70	.08	1.10	4.26	1.02	21.68
CW94		6/11/55	2154	7.7	931	.32	19.6	1.7			12.08	10.64	5.57	.09	.77	3.76	1.08	22.77
CW102		7/10/56	1990	7.9	1730	.30	13.3	1.0			12.67	8.90	3.33	.06	.00	4.95	.72	19.29
CW132		8/7/56	1842	8.1	1592	.30	11.2	.8			12.25	7.88	2.55	.05	.23	4.60	.58	17.32
CW161		9/17/56	1991	7.9	1796	.24	10.7	.8			13.08	9.36	2.70	.06	.26	4.71	.57	19.66
Station 11-L South Fork of Naples Drain above Confluence with North Fork																		
CW68		11/7/55	5538	7.7	6300	1.08	27.3	-			23.04	38.27	23.10	.18	.69	5.76	3.56	74.58
CW95		6/11/56	3442	7.9	3388	.81	24.6	2.8			14.68	20.97	11.67	.20	.67	4.28	1.55	41.02
CW104		7/10/56	5917	8.0	6468	1.26	27.6	4.3			24.58	40.10	24.75	.20	.33	7.04	3.31	78.95
CW131		8/7/56	6595	8.0	7508	1.7	29.1	5.0			26.67	45.00	29.47	.23	.45	6.90	4.24	89.78
CW162		9/17/56	5877	7.9	6510	1.20	22.8	3.6			34.92	40.10	22.20	.17	1.37	5.49	2.65	87.88
Equivalent Weights											200	12.2	230	39.1	300	61.0	35.5	480

WATER ANALYSIS  
REGIONAL LABORATORY

Table 17 (cont.)

Vernal Unit

Collected by \_\_\_\_\_ Date \_\_\_\_\_ Project \_\_\_\_\_

Submitted by \_\_\_\_\_ Date \_\_\_\_\_ Location \_\_\_\_\_

Analyzed by \_\_\_\_\_ Date \_\_\_\_\_

Lab. No	Field No.	Sampling Date	EC x 10 <sup>6</sup> @ 25° C	pH	Total dissolved salts ppm	Boron ppm	% Sodium	Sodium Adsorption Ratio	Residual Carbonates	Equivalents per million or milliequivalents per liter								
										Cations				Anions				
										Ca	Mg	Na	K	CO <sub>3</sub>	HCO <sub>3</sub>	Cl	SO <sub>4</sub>	
Station 11-Ma Naples Drain 1/2 mile east of U.S. Highway 40																		
CW64		11/7/55	3053	7.6	3064	.60	12.3	1.2		21.83	16.17	5.34	.08	.51	4.74	.99	37.18	
CW77		3/13/56	1977	8.3	1736	.24	14.4	1.1		12.36	9.19	3.64	.07	.88	3.69	.84	19.85	
Station 11-Mb Ashley Central Canal at Naples																		
CW65		11/8/55	1380	7.7	1144	.14	6.7	.4		9.94	5.61	1.13	.08	.40	3.82	.21	12.33	
CW78		3/13/56	771	8.1	567	.01	5.2	.2		5.02	3.21	.45	.04	.45	3.24	.17	4.86	
CW90		5/25/56	500	8.4	353	.06	11.7	.3		2.92	1.64	.61	.05	.06	1.40	.09	3.67	
CW108		7/10/56	421	7.9	293	.04	6.6	.2		2.71	1.38	.29	.02	.00	2.18	.01	2.21	
CW127		8/7/56	467	8.4	313	.03	5.7	.2		2.74	1.65	.27	.04	.17	2.29	.04	2.20	
CW160		9/18/56	795	7.7	602	.14	6.0	.3		4.78	3.33	.52	.05	None	.66	.09	7.93	
Equivalent weights										20	12	23	39	30		35.5	48	



WATER ANALYSIS  
REGIONAL LABORATORY

Table 17 (cont.)

Vernal Unit

Collected by \_\_\_\_\_ Date \_\_\_\_\_ Project \_\_\_\_\_

Submitted by \_\_\_\_\_ Date \_\_\_\_\_ Location \_\_\_\_\_

Analyzed by \_\_\_\_\_ Date \_\_\_\_\_

Lab No	Field No	Sampling Date	EC x 10 <sup>3</sup> @ 25°C	pH	Total dissolved salts ppm	Boron ppm	% Sodium	Sodium Adsorption Ratio	Residual Carbonates	Equivalents per million or milliequivalents per liter								
										Cations				Anions				
										Ca	Mg	Na	K	CO <sub>3</sub>	HCO <sub>3</sub>	Cl	SO <sub>4</sub>	
<u>Station 11-Na Ashley Creek above Naples Drain</u>																		
CW32		7/21/55	2082	8.0	1812	.60	14.9	1.2		10.65	11.89	3.98	.08	.48	4.51	.52	20.96	
CW48		9/26/55	2123	8.0	1848	.52	14.3	1.1		11.34	11.79	3.88	.10	.67	5.02	.65	21.45	
CW97		6/11/56	1510	7.7	1242	.45	16.1	1.1		8.07	7.35	2.97	.10	.00	5.51	.37	12.61	
<u>Station 11-Nb Seep Area <math>\frac{1}{4}</math> mile North of Naples Drain and <math>\frac{1}{4}</math> mile West of Ashley Creek</u>																		
CW61		11/8/55	7384	7.5	5236	1.11	22.4	-		25.19	31.02	16.35	.33	1.24	6.91	1.14	63.60	
CW96		6/11/56	4547	7.5	4876	.90	18.4	2.4		26.29	30.00	12.84	.53	.76	6.71	1.14	61.05	
<u>Station 11-Nc Naples Drain at Mouth</u>																		
CW51		9/26/55	3428	8.1	3398	.34	14.9	1.5		21.12	20.41	7.29	.13	.96	4.38	1.29	42.32	
CW110		7/10/56	3415	7.9	3438	.80	14.7	1.6		22.07	18.37	6.96	.08	.00	4.99	1.16	41.33	
CW133		8/7/56	3740	8.1	3794	.79	17.4	2.0		21.65	21.07	9.02	.14	.00	3.52	1.31	47.05	
CW156		9/18/56	3692	7.7	2826	.76	15.8	1.7		23.38	19.39	8.02	.11	.90	4.48	1.29	44.23	
																185	480	

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WATER ANALYSIS  
REGIONAL LABORATORYTable 17 (cont.)  
Vernal Unit

Collected by \_\_\_\_\_ Date \_\_\_\_\_ Project \_\_\_\_\_

Submitted by \_\_\_\_\_ Date \_\_\_\_\_ Location \_\_\_\_\_

Analyzed by \_\_\_\_\_ Date \_\_\_\_\_

Lab No	Field No	Sampling Date	ECx10 <sup>6</sup> @ 25°C	pH	Total dissolved salts ppm	Boron ppm	% Sodium	Sodium Adsorption Ratio	Residual Carbonates	Equivalents per million or milliequivalents per liter							
										Cations				Anions			
										Ca	Mg	Na	K	CO <sub>3</sub>	HCO <sub>3</sub>	Cl	SO <sub>4</sub>
Station 12-M 3 Miles above	Slaugh Gulch Mouth																
CW80		3/13/56	597	8.2	413	.01	7.4	.3		3.60	2.26	.47	.04	.50	2.19	.14	3.54
CW91		5/25/56	715	8.2	537	.12	11.1	.3		4.29	2.66	.87	.05	.07	1.76	.13	5.91
CW107		7/10/56	1171	7.8	940	.21	11.5	.6		7.66	4.29	1.56	.05	.00	2.92	.28	10.36
CW123		8/7/56	630	8.1	443	.05	8.5	.3		4.01	1.90	.55	.03	.00	2.47	.11	3.91
CW154		9/17/56	2675	7.7	2608	.78	15.5	1.4		18.11	10.87	5.34	.07	.87	3.85	.82	29.65
Station 13-P at River Canal	Ashley Creek Diversion																
CW53		9/27/55	2969	8.0	2788	.27	20.3	2.3		15.12	16.73	8.16	.15	.99	3.92	1.08	34.17
CW98		6/11/56	2033	7.8	1776	.44	18.1	1.4		10.83	10.20	4.68	.13	.00	5.17	.63	20.04
CW106		7/10/56	2526	8.2	2348	.54	21.1	1.9		11.62	13.57	6.76	.13	.23	2.28	.88	28.69
Equivalent Weights										200	122	230	39	300	610	355	480



WATER ANALYSIS  
REGIONAL LABORATORYTable 17 (cont.)  
Vernal Unit

Collected by \_\_\_\_\_ Date \_\_\_\_\_ Project \_\_\_\_\_

Submitted by \_\_\_\_\_ Date \_\_\_\_\_ Location \_\_\_\_\_

Analyzed by \_\_\_\_\_ Date \_\_\_\_\_

Lab No	Field No	Sampling Date	EC x 10 <sup>6</sup> @ 25°C	pH	Total dissolved salts ppm	Boron ppm	% Sodium	Sodium Adsorption Ratio	Residual Carbonates	Equivalents per million or milliequivalents per liter										
										Cations				Anions						
										Ca	Mg	Na	K	CO <sub>3</sub>	HCO <sub>3</sub>	Cl	SO <sub>4</sub>			
<u>Station 15-R Ashley Creek near Jensen (at Gaging Station)</u>																				
CW31		7/21/55	5366	8.0	5504	1.5	37.2	-			17.17	29.69	27.86	.27	1.09	3.70	3.34	65.98		
CW54		9/27/55	4235	8.1	4256	.66	32.7	4.5			14.73	23.83	18.81	.22	.61	2.97	2.21	51.80		
CW74		3/14/56	2000	8.1	1668	.32	17.8	1.4			11.33	9.57	4.54	.09	.74	4.42	.78	19.59		
CW88		5/25/56	581	8.1	413	.09	15.4	.5			3.05	2.15	.96	.06	.06	2.22	.14	3.80		
CW111		7/10/56	3555	8.1	3438	.77	27.2	3.1			15.05	19.49	12.99	.21	.16	5.13	1.63	40.82		
CW128		8/7/56	5300	8.1	5500	1.0	33.4	5.0			18.06	30.82	24.66	.25	.11	5.14	3.25	65.29		
CW157		9/18/56	4385	7.9	4584	1.11	30.6	7.9			16.32	25.51	18.55	.21	.87	4.16	2.27	53.29		
<u>Station 16-S Union Canal at USBR Gage</u>																				
CW105		7/10/56	3772	7.7	3672	.96	29.6	3.6			15.55	20.05	15.08	.22	.00	5.43	1.87	43.60		
Equivalent Weights											200	122	230	391	300	610	355	480		

### Water Rights

Water for the Vernal Unit, along with other increased uses of water of the Colorado River Basin, is subject to limitations imposed by the Colorado River Compact and the Upper Colorado River Basin Compact. It is further subject to the Utah State water law, which is the law of appropriation.

#### Colorado River Compact

The Green River, a tributary of the Colorado River, is subject to the terms of the Colorado River Compact. This compact provides principally for an apportionment from the Colorado River System in perpetuity to the upper basin and to the lower basin, respectively, the exclusive beneficial consumptive use of 7,500,000 acre-feet of water per annum. Articles II and III of the compact are quoted below for information.

#### ARTICLE II

"As used in this compact:

"(a) The term 'Colorado River System' means that portion of the Colorado River and its tributaries within the United States of America.

"(b) The term 'Colorado River Basin' means all of the drainage area of the Colorado River system and all other territory within the United States of America to which the waters of the Colorado River system shall be beneficially applied.

"(c) The term 'States of the upper division' means the States of Colorado, New Mexico, Utah, and Wyoming.

"(d) The term 'States of the lower division' means the States of Arizona, California, and Nevada.

"(e) The term 'Lee Ferry' means a point in the main stream of the Colorado River 1 mile below the mouth of the Paria River.

"(f) The term 'Upper Basin' means those parts of the States of Arizona, Colorado, New Mexico, Utah, and Wyoming within and from which waters naturally drain into the Colorado River system above Lee Ferry, and also all parts of said States located without the drainage area of the Colorado River system which are now or shall hereafter be beneficially served by waters diverted from the system above Lee Ferry.

"(g) The term 'Lower Basin' means those parts of the States of Arizona, California, Nevada, New Mexico, and Utah within and from which waters naturally drain into the Colorado River system below Lee Ferry, and also all parts of said States located without the drainage area of the Colorado River system which are now or shall hereafter be beneficially served by the waters diverted from the system below Lee Ferry.

"(h) The term 'domestic use' shall include the use of water for household, stock, municipal, mining, milling, industrial, and other like purposes, but shall exclude the generation of electrical power."

### ARTICLE III

"(a) There is hereby apportioned from the Colorado River system in perpetuity to the upper basin and to the lower basin, respectively, the exclusive beneficial consumptive use of 7,500,000 acre-feet of water per annum, which shall include all water necessary for the supply of any rights which may now exist.

"(b) In addition to the apportionment in paragraph (a), the lower basin is hereby given the right to increase its beneficial consumptive use of such waters by 1,000,000 acre-feet per annum.

"(c) If, as a matter of international comity, the United States of America shall hereafter recognize in the United States of Mexico any right to the use of any waters of the Colorado River system, such waters shall be supplied first from the waters which are surplus over and above the aggregate of the quantities specified in paragraphs (a) and (b); and if such surplus shall prove insufficient for this purpose, then the burden of such deficiency shall be equally borne by the upper basin and the lower basin, and whenever necessary the States of the upper division shall deliver at Lee Ferry water to supply one-half of the deficiency so recognized in addition to that provided in paragraph (d).

"(d) The States of the upper division will not cause the flow of the river at Lee Ferry to be depleted below an aggregate of 75,000,000 acre-feet for any period of 10-consecutive years reckoned in continuing

progressive series beginning with the first day of October next succeeding the ratification of this compact.

"(e) The States of the upper division shall not withhold water, and the States of the lower division shall not require the delivery of water, which cannot reasonably be applied to domestic and agricultural uses.

"(f) Further equitable apportionment of the beneficial uses of the waters of the Colorado River system unapportioned by paragraphs (a), (b), and (c) may be made in the manner provided in paragraph (g) at any time after October 1, 1963, if and when either basin shall have reached its total beneficial consumptive use as set out in paragraphs (a) and (b).

"(g) In the event of a desire for further apportionment as provided in paragraph (f) any two signatory States, acting through their governors, may give joint notice of such desire to the governors of the other signatory States, and to the President of the United States of America, and it shall be the duty of the governors of the signatory States and of the President of the United States of America forthwith to appoint representatives, whose duty it shall be to divide and apportion equitably between the upper basin and lower basin the beneficial use of the unapportioned water of the Colorado River system as mentioned in paragraph (f), subject to the legislative ratification of the signatory States and the Congress of the United States of America."

#### Upper Colorado River Basin Compact

Use of apportioned water in the upper basin is subject to certain restrictions, not definable at this time, to assure required deliveries to the lower basin and Mexico. Were it not for these restrictions, Utah's apportioned use would amount to 1,713,500 acre-feet annually or 23.0 percent of 7,450,000 acre-feet (the 7,500,000 acre-feet annually apportioned the upper basin less 50,000 acre-feet apportioned to Arizona). Utah's present uses measured in terms of long-time average depletions at Lee Ferry have been estimated by the Engineering Advisory Committee to the Upper Colorado River Basin Compact Commission to total 544,300 acre-feet annually for the period 1914-1945. Studies indicate anticipated depletions of about 83,700 acre-feet annually, which includes increases in depletions occurring since 1945 and anticipated use of water by authorized projects not yet constructed. Utah's total present and authorized uses, therefore, amount



to 628,000 acre-feet annually. Utah's uses can thus be increased approximately 1,085,500 acre-feet annually in the future. The average needs of the Central Utah project have been estimated at 746,300 acre-feet annually at the sites of use, and there would remain for other uses about 339,200 acre-feet.

The water supply for the Vernal Unit is only a very small part of Utah's use apportioned under this compact.

#### Utah State water law

Since 1903 when the Utah water law was enacted, water has been considered the property of the public, and rights to its use can be acquired only by actual diversion and application to beneficial use. Priority of a right is determined by the date of its initiation and "first in time is first in right."

Water rights may be initiated only by filing appropriate applications with the State Engineer and proceeding in accordance with specific statutory provisions. If there is unappropriated water in the source of supply described in an application to appropriate water, and if the use of water can be made under a feasible plan which will not prove detrimental to the public welfare and will not impair existing rights or interfere with a more beneficial use of water, it is the duty of the State Engineer to approve application. The approval of an application gives the applicant the right to proceed with the construction of the necessary works and to use the water in order of priority. When the necessary works are constructed and water is applied to beneficial use as contemplated by the application, written proof thereof must be filed with the State Engineer. Upon approval of the proof of beneficial use of water, the State Engineer issues a certificate of appropriation which is prima facie evidence of the water right therein described.

#### Existing Rights

In November 1897, the waters of Ashley Creek were adjudicated in the Fourth Judicial District of the State of Utah as a result of a lawsuit involving the Ashley Central Irrigation Company, the Rock Point Irrigation Company, the Ashley Upper Irrigation Company, the White Wash and Spring Creek Irrigation and Canal Company, and numerous individuals. This decree allotted to each canal company and each individual the right to the use of a certain portion of all the waters of Ashley Creek. Individuals were given the right to have their water delivered through the canals of the irrigation companies, if they so desired, by paying their proportionate share of operation and maintenance costs. The following tabulation gives a summary of the percent of Ashley Creek flow decreed to water users served by each canal system. These rights have been more or less voluntarily restricted by the water users to

## WATER SUPPLY

## WATER RESOURCES

apply only to the first 500 second-feet in Ashley Creek. Water in the stream in excess of the 500 second-feet has thus been made available to appropriation.

<u>Owner</u>	<u>Percent of flow</u>
Ashley Upper Canal	36.3
Ashley Central Canal	33.5
Rock Point Canal	19.8
Island Ditch	7.4
Stanaker Ditch	2.0
Dodds Ditch	1.0
Total	100.0

Other court decrees have been made on Ashley Creek at various times since 1897, most of them for seepage water which would not be affected by the project. Several of the decrees concern parties who would be participants in the Vernal unit, and their rights would be modified by contracts as explained under the section "Vernal Unit Rights." Table 18 summarizes court decrees for water in Ashley Valley with a short explanation of each.

Appropriations of Ashley Creek waters since 1903 are primarily for storage, and for use of seepage and flood flows. Table 19 lists applications by the Mosby Irrigation Company for a proposed diversion from the headwaters of Dry Fork Creek to Mosby Creek, which is tributary to Deep Creek and Uinta River. If these diversions are made, they will reduce the water available to the Vernal unit by a small amount. The diversions would not appreciably affect the Vernal unit due to the small amount of water that Dry Fork contributes to the streamflow.

Applications having their point of use upstream from Vernal unit area are listed in Table 20. Consumptive use resulting from these rights has already been reflected in the streamflow records of Ashley Creek at "Sign of the Maine" gage, and these rights will not affect the project.

Table 21 lists the applications which would be incorporated into the Vernal unit along with the users under the court decree of 1897. Whether the water rights are based on court decrees or on applications, these claimants to flows of Ashley Creek would benefit from construction of the Vernal unit. Application No. 4796 was made by the Highline Canal Company, which operates one of the six large canals serving the Vernal unit area. Application numbers 10,053 and 11,708 are for storage of Brush Creek water in Oaks Park Reservoir with later diversion to Ashley Creek. These rights are subsequent to natural flow rights of earlier priority on Brush Creek.

Table 18

## Water Decrees on Ashley Creek

Civil Case No.	Case	Date of Decree	Present Owners	Amount	Remarks
18	E. G. DeFriez et al vs Sterling D. Colton et al	Nov. 17, 1897	Ashley Upper, Ashley Central & Rock Point Canal; Island Ditch, Stanaker Ditch, Dodds Ditch, Colton Ditch, Hardy Ditch	All the water of Ashley Creek at mouth and in South Fork	This suit was insituted by the plain- tiffs to restrain the elected officers of Uintah County from administering the waters of Ashley Creek. Also they wished the court to quiet title to the waters of said stream and write a decree setting forth the rights to the use of water in Ashley Creek. This decree dis- tributed to canals and individual owners all the waters of Ashley Creek at its mouth and that which seeps into or rises in the South Fork.
67	Richard James et al vs William Gibson et al	Nov. 11, 1897	Private owners in the Spring Creek and Gibson Slough	All the waters of Spring Creek and Gibson Slough	This suit was brought into court for the express purpose of quieting title to the use of waters of Spring Creek, a tribu- tary of Ashley Creek. This was con- sidered a friendly suit and as a result of the filing of the complaint and an- swer to the same decree was written ad- judicating the waters of Spring Creek.
127-483	Ashley Central Irri- gation Co. et al vs Moroni Pitt et al	1912- 1915	Stanley Jones Loren Pitt, Moroni Pitt, William Warby, Lulu N. Pitt, Reva McKee and Rex and Everett Pitt	2 Second- feet	The plaintiff attempted to prohibit the defendant from diverting excessive quan- tities of water from Ashley Creek. The defendants claiming a right dating to 1900 were granted a flow of 2 second-feet after prior use requirements were satis- fied.

Table 18 (Cont.)

Civil Case No.	Case	Date of Decree	Present Owners	Amount	Remarks
637	J. T. Rasmussen, Union Canal Co. et al vs Joseph Paulson	Dec. 1913	Union Canal Co.	Sufficient water to irrigate 800 acres	This case was a friendly suit brought into court to quiet title to the water rights of the Union Canal Co. This decree and application No. 2428 to appropriate water cover the same rights.
761	River Irrigation Co. vs Union Canal Co. et al	Dec. 1915	River Irrigation Company and Union Canal Co.	River Irrigation Co. 5 5/7 S.F. Union Canal Co. 10 6/7 S.F. Jensen Irrig. Co. 15 S.F. (Secondary high water right)	This suit was brought into court to quiet title to the waters of lower Ashley Creek. The decree differed from others because it specified definite quantities of water. Applications 2428 and 4041 to appropriate water and decree written in the case of J. T. Rasmussen et al vs. Joseph Paulson are covered by this decree.
795	O. D. Allen et al vs Ashley Central Irrigation Co.	June 1916	J. M. Allen S. G. Oaks et al	1 second- foot per 70 acres or 44/79 of a second- foot	This suit was instituted by the plaintiffs in order to establish their rights as against those of the Ashley Central Irrigation Co. The defendants and plaintiffs comingled their water as a result of a change in the point of diversion by the canal company. Later differences arose as to the amount the plaintiffs were entitled to divert from the joint canal. The canal company defaulted and the Court granted the plaintiffs a definite quantity of water for each acre of land.



Table 18 (Cont.)

Civil Case No.	Case	Date of Decree	Present Owners	Amount	Remarks
806	Nillie Fagan vs Micheal O. Nash et al	October 1916	Jose H. Karren et al	Variable	This suit was instituted by the heirs of Charles E. Fagan for the express purpose of quieting title to the seepage water arising in Lybbert Gulch, located in the southern part of Ashley Valley.
839	George M. Hathaway vs Willard Williams et al	March 1919		Variable	Court action was taken by the plaintiff for the purpose of quieting title to seepage water flowing in Slauch Gulch in southern Ashley Valley. Seepage water was prorated on the basis of stock owned in the Ashley Upper Canal Company, because the Gulch was also used as a lateral by the stock holders.
848	E. J. Young et al vs Geo. H. Southam	Dec. 1917		Variable	This action was brought into Court to quiet title to seepage water rising in an unnamed draw in the southern part of Ashley Valley first south of Slauch Gulch. Water prorated to the various claimants on the basis of their holdings.
	Vernal City, a municipal corp. vs Ashley Upper Irrigation Co.	June 1920	Vernal City	15-99/320 shares in Ashley Central Canal Co.	Court action was taken by Vernal City for the purpose of changing the point of diversion of their domestic water supply. The supply was taken directly from the canal and was being contaminated by return flow from irrigated lands above the canal. A decree was written granting the city the right to change their diversion to a point on Ashley Creek above all irrigation diversions.

Table 18 (Cont.)

Civil Case No.	Case	Date of Decree	Present Owners	Amount	Remarks
960	Thomas Mantle et al vs John B. Eaton et al	June 1920		Variable	This suit was instituted by the plaintiffs for the purpose of quieting title to the seepage water rising along the McNaughton Gulch in the western part of Ashley Valley. A decree was written granting a percentage of the water to each of the plaintiffs and defendants.

Table 19  
ABSTRACT OF WATER RIGHT APPLICATIONS  
ON DRY FORK FOR TRANSBASIN DIVERSION TO MOSBY CREEK

File No.	Applicant	Use	Sec. Ft.	Acre Feet	Point of Diversion	Period of Use	Area to be Irrig. or Horse-power	Priority	Source	Remarks
11795	Mosby Irr. Co.	Irr	100		8250 ft. N. & 1133 ft. E. of the W $\frac{1}{4}$ Cor. Sec. 5, T. 3 N. R. 1 E., USB&M	4/1-11/1	8,527.45	11/18/38	Dry Fork Creek	Proof due 9/20/56 <sup>1/</sup>
11796	Mosby Irr. Co.	Irr		500	8325 ft. N. & 1285 ft. E. of the W $\frac{1}{4}$ Cor. Sec. 5, T. 3 N., R. 1 E. USB&M	7/1-12/1 Store 11/1-7/1	4,880	1/6/36	Dry Fork Creek	Proof due 9/20/56 <sup>1/</sup>
11797	Mosby Irr. Co.	Irr	50		7850 ft. N. & 6357 ft. E. of W $\frac{1}{4}$ Cor. Sec. 5, T. 3 N. R. 1 E., USB&M	4-1-11/1	4,880	11/18/38	North Fork of Dry Fork	Proof due 9/20/56 <sup>1/</sup>
11798	Mosby Irr. Co.	Irr		1,000	9670 ft. N. & 7012 ft. E. of W $\frac{1}{4}$ Cor. Sec. 5, T. 3 N., R. 1 E. USB&M	7/1-12/1 Store 11/1-7/1	8,527.45	8/1/35	North Fork of Dry Fork	Proof due 9/20/49 <sup>1/</sup>

<sup>1/</sup> Extension of time protested. No decision to date by Utah State Engineer.



Table 20  
ABSTRACT OF WATER RIGHT APPLICATIONS  
ON ASHLEY CREEK AND DRY FORK FOR USE OR STORAGE UPSTREAM FROM PROJECT AREA

File No.	Applicant	Use	Amount Sec.Ft. Ac.Ft.	Point of Diversion	Period of use	Area to be irrigated or horsepower	Priority	Source	Remarks
1227	Vernal Milling & Light	Power	55	N 33°21' W. 584.4 ft. from W 1/4 Cor. Sec. 7, T. 3 S., R. 21 E., SLB&M	1/1-12/31	600 hp	3/4/07	Ashley Creek	Cert. # 71B
2237	Isabell M. Hacking	Irr	.28	N 42°5' W. 529 ft. from E 1/4 Cor. Sec. 12, T. 3 S., R. 20 E., SLB&M	9/1-7/10	19.6	1/26/09	Ashley Creek	Cert. #491
6026	Murley A. Massey	Irr	0.26	N. 12°55' W. 1041.9 ft. from SE Cor. Sec. 8, T. 3 S., R. 20 E. SLB&M	5/1-9/30	18	7/3/25	Dry Fork	Cert. #1556
6081	Massey, Ira D. etal	Irr	.85	N. 45°50' W. 932 ft. from E 1/4 Cor. Sec. 25, T. 3 S., R. 20 E. SLB&M	4/1-11/1	58.4	3/1/15	Dry Fork	Cert. #950
6795	Francis M. Caldwell	Irr	.36	N. 72°33' W. 480 ft. from N 1/4 Cor. Sec. 29, T. 3 S., R. 2 W., USB&M	4/1-11/1	25.1	6/8/16	Dry Fork	Cert. No. 1483
8250	Ashley Res. Co.	Irr	2530	Storage Res. at points (1) S 52°45' W. 24,200 ft; (2) S 56°45' W. 22380, ft., (3) S. 51°15' W. 22,000 ft. (4) S 6°30' E. 12,045 ft. (5) S. 79°20' W. 35,100 ft. all from NW Cor. T.1 S., R.20 E., SLB&M	Stored 1/1-12/31	44,800	11/23/35		Extension granted to 12/19/58
8755	Ashley Res. Co.	Irr	14 1425	(1) S. 75°50' W. 27,456 ft; (2) N 87°44' W., 11,458 ft., both from SW cor. Sec. 36 T. 1 N., R. 19 E., SLB&M	5/1-10/1 Stored 1/1-12/31	44,800	11/23/35		Extension granted to 12/19/58
10022	George H. Luck	Irr	1	S. 6°00' W. 1050 ft. from NE Cor. Sec. 31, T. 3 S., R. 21 E. SLB&M	4/1-6/30	80	7/21/26	Ashley Creek	Cert. #2427
11143	High Line Canal Co.	Irr	15 180	2150 ft. W. & 2440 ft. N. from NW Cor. Sec. 1, T. 1 S., R. 19 E. SLB&M	4/15-11/15 Stored 1/1-12/31	6,000	9/14/38	Unnamed Trib. of Ashley Cr.	Extension granted to 5/11/60
13322	Utah Power & Light	Power	55	N. 41°15' W. 530 ft. from E 1/4 Cor. Sec. 12 T. 3 S., R 20 E. SLB&M	10/15-4/15	2,400 hp	2/9/40	Ashley Creek	Pending cert. Proof submitted



Table 21  
ABSTRACT OF WATER RIGHT APPLICATIONS  
ON ASHLEY CREEK AND BRUSH CREEK WHICH WOULD BE INCORPORATED INTO VERNAL UNIT

File No.	Applicant	Use	Amount Sec.Ft. Ac.Ft.	Point of Diversion	Period of use	Area to be irrigated or horsepower	Priority	Source	Remarks
1932	Allen, O. D. etal	Irr	.36	N. 59°42' W. 747 ft. from E 1/4 Cor. Sec. 8, T. 4 S., R. 21 E., SLB&M	3/1-12/1	19.25	3/18/12	Ashley Creek	Cert. # 633
4796	Highline Canal Co.	Irr	182	N. 13°10' E. 1782 ft. from NW Cor. Sec. 5, T. 4 S., R. 21 E. SLB&M	3/1-10/31	13419.47	8/2/12	Ashley Creek	Pending
5156	Morrison, Jane B. etal	Irr	4-2/7	1386 ft. S. & 363 ft. E. of the N 1/4 Cor. Sec. 14 T. 4 S. R. 21 E. SLB&M	3/15-7/10	250	7/30/13	Ashley Creek	Cert. #479
5985	W. D. Newton	Irr	2.5	N 5° E. 1020 ft. from S 1/4 Cor. Sec. 5, T. 4 S., R. 21 E. SLB&M	3/1-11/1	175.8	12/3/14	Ashley Creek	Cert. #1147
a-532	Vernal Irr. Co.	Irr		N. 26°15' W. 1940 ft. from the N 1/4 Cor. Sec. 8, T. 4 S., R. 21 E., SLB&M	2/9/20		2/9/20	Ashley Creek	Certificate issued Segreg. of Appl. 4796
a-1548	Vernal City	Irr	5	N. 58°30' E. 1570 ft. from the SW Cor. Sec. 19, T.3 S., R.21 E., SLB&M	1/1-12/31		9/4/40	Ashley Creek	Change on decreed right. Extension granted to 5/28/58 Extension requested Feb. 13, 1957
10053	Resettlement Adm.	Irr	4,500	N. 24° E. 1350 ft. from SW Cor. Sec. 32, T. 3 S., R.21 E. SLB&M	6/1-10/30 Store 1/1-12/31	10,700	12/12/28	Brush Creek	
11708	Resettlement Adm.	Irr.	3,000	Center of dam is located at a point S. 56° W. 1300 ft. from E 1/4 Cor. Sec. 12, T. 1 S., R. 20 E. SLB&M	3/1-12/1 Store 1/1-12/31	20,000	3/1/35	Brush Creek	Water is conveyed to and distributed from Ashley Cr. Extension granted to Dec.6,1957
11303	Highline Canal Assigned to Vernal City		500	1000 ft. N. and 3900 ft. W of NW Cor. Sec. 1 T. 1 S. R. 19 E. SLB&M			1/5/44		Proof due Jan.20,1958
24219	Vernal City		5				9/25/52		Diversion from Ashley Creek Spring Application approved Nov. 28, 1956. Proof due Jan. 25, 1959

Table 22  
ABSTRACT OF WATER RIGHT APPLICATIONS  
FROM TRIBUTARIES OF ASHLEY CREEK IN ASHLEY VALLEY

File No.	Applicant	Use	Sec. Ft.	Acre Feet	Point of Diversion	Period of Use	Area to be Irrig. or Horse-power	Priority	Source	Remarks
5201	Water Simper	Irr	.67		(1) 1320 ft. E. & 440 ft. N. from SW Cor. Sec. 9, (2) 570 ft. N. & 769 ft. E. of W $\frac{1}{4}$ Cor. Sec. 9, T. 5 S., R. 22 E., SLB&M	4/1-12/1	46.6	5/9/13	2 Unnamed draws	Cert. #869
6800	Hyrum E. Seeley	Irr	.25		N. 62°05' E. 2087 ft. from SW Cor. Sec. 12, T. 4 S., R. 21 E., SLB&M	4/1-11/1	5.5	6/10/16	Unnamed draw	Cert. #918
7376	G. H. Southam	Irr	.75		S. 41°40' E., 1330 ft. from NW Cor. Sec. 10, T. 5 S., R. 22 E., SLB&M	4/1-11/1	80 Ac.	6/23/17	Unnamed draw	Cert. #998

Table 23  
 ABSTRACT OF WATER RIGHT APPLICATIONS  
 FROM ASHLEY CREEK DOWNSTREAM FROM VERNAL UNIT AREA

File No.	Applicant	Use	Sec. Ft.	Acre Feet	Point of Diversion	Period of Use	Area to be Irrig. or Horse- power	Priority	Source	Remarks
2428	John T. Rasmussen et al	Irr	8		N. 85°34' W. 1294 ft. from E $\frac{1}{4}$ Cor. Sec. 23, T. 5 S. R. 22 E., SLB&M	4/1-9/1	642.36	5/11/09	Ashley Creek	Cert. #135 B
4041	Jensen Irr. Co.	Irr	14.4		N. 44°50' W. 2899 ft. from E $\frac{1}{4}$ Cor. Sec. 10, T. 5 S., R. 22 E., SLB&M	4/1-7/15	1023.4	6/5/11	Ashley Creek	Cert. #1083
a-509	Jensen Irr. Co.	Irr			N. 42°38' W. 2332 ft. from E $\frac{1}{4}$ Cor. Sec. 10, T. 5 S., R. 22 E., SLB&M	10/20/19		10/20/19	Ashley Creek	#1083 - Segregation of Appl. 4041



Three applications have been filed for water from tributaries to Ashley Creek, a considerable distance downstream from the head of Ashley Valley (see Table 22). These applications would have no effect on Vernal unit.

Table 23 lists water right applications for water from Ashley Creek downstream from the Vernal unit area. The source of supply to these applicants is return flow water from presently irrigated lands in the Vernal unit area. Water rights for Vernal unit would not be adversely affected by these lower rights. Vernal unit may be expected to improve the supply to these applicants by providing increased late-season water from return flow.

#### Vernal Unit Rights

Present practices are to divert the Ashley Creek flow into the major canals at all times of the year. Winter flows are diverted through the canals for stock watering purposes, and high spring flows are diverted to the maximum capacity of the canals, which are in excess of the diversions necessary under normal reasonably efficient irrigation practices. Local water users recognize that present practices must be modified considerably if sufficient water is to be made available for operation of the Vernal unit. Meetings of water users of the Rock Point Irrigation Company, Ashley Central Irrigation Company, Ashley Upper Irrigation Company, Highline Irrigation Company, Island Ditch Company, and Dodds Ditch Company have resulted in the signing of Resolutions as follows:

#### RESOLUTION

"Be it resolved by the Board of Directors of \_\_\_\_\_  
\_\_\_\_\_, as follows:

"WHEREAS, at a meeting of the Board of Directors duly called and held \_\_\_\_\_, 1956, at the director's room of the Uintah State Bank, Vernal, Utah, at \_\_\_\_\_ p.m., a proposed form of contract between the United States and the Company relating to exchange and adjustment of water rights of the Company under the proposed Vernal unit of the Central Utah project to be constructed by the United States, was presented and considered.

"NOW, THEREFORE, the President and Secretary of the Company are hereby authorized to execute on behalf of the Company, the form of contract so presented with the changes reflected on the copy of the form, and it being understood that the form of contract with such changes is subject to further review and final approval as to form by the United States, the President and Secretary are further authorized and empowered to execute and deliver



to the United States the form of contract as it may be revised after such review provided such revisions are satisfactory to the President and approved by the Company attorney."

CERTIFICATE

"I, \_\_\_\_\_, Secretary of the \_\_\_\_\_ Company, do hereby certify that the foregoing resolution of the Board of Directors of the Company was passed by said board at a special meeting duly called and held on \_\_\_\_\_. I further certify that there is a total of \_\_\_\_\_ directors in the Company, and that \_\_\_\_\_ directors voted in favor of the foregoing resolution and \_\_\_\_\_ directors voted against the foregoing resolution."

\_\_\_\_\_  
Secretary of the

\_\_\_\_\_  
Company

The form of contract contemplated by the Resolutions is to be executed between the United States and each of the six major canal companies. It relates to the exchange and adjustment of the decreed water rights. These contracts restrict the use of water under the decrees to a stipulated annual irrigation demand properly distributed by months. The water in Ashley Creek in excess of these monthly requirements is considered divertible by the unit. The annual irrigation demand set out in the contracts is for the purpose of defining the maximum permissible use of water under the decrees only, and is not to be confused with the ideal annual irrigation demand used in the operation studies to determine the demand for Vernal unit water. A blank form of contract is included as follows:

UNITED STATES  
DEPARTMENT OF THE INTERIOR  
BUREAU OF RECLAMATION  
CENTRAL UTAH PROJECT, UTAH  
VERNAL UNIT

CONTRACT BETWEEN THE UNITED STATES  
AND \_\_\_\_\_ COMPANY  
RELATING TO EXCHANGE AND ADJUSTMENT OF WATER RIGHTS

THIS CONTRACT, made this \_\_\_\_\_ day of \_\_\_\_\_, 1957, in pursuance of the Act of June 17, 1902 (32 Stat. 388) and acts amendatory thereof or supplementary thereto, particularly the Act of April 11, 1956 (Public Law 485, 84th Congress), between THE UNITED STATES OF AMERICA, herein styled the United States, acting in this behalf by the Regional Director, Region 4, Bureau of Reclamation, Department of the Interior, hereinafter referred to as the Regional Director, and the \_\_\_\_\_ COMPANY, a corporation organized under the laws of the State of Utah with its place of business at Utah, herein styled the Company:

WITNESSETH, That:

2. WHEREAS, the United States is planning the construction of the Vernal unit as part of the Central Utah Project, herein styled unit, including facilities which will be used, among other purposes, to deliver water into the Company canal, commonly known as the \_\_\_\_\_ Canal, permitting an exchange of Company water as hereinafter described for unit water delivered from the proposed Steinaker Reservoir.

3. WHEREAS, the Company has certain water rights in the flow of Ashley Creek as referred to in that certain decree entered by Judge Warren H. Dusenberry on November 17, 1897, Uintah County Case No. 18, and

4. WHEREAS, in order that there will be adequate water supply for the unit, it is necessary to establish the amount of water that the Company will receive under the above-mentioned decree when said unit is constructed and in operation.

5. NOW, THEREFORE, in consideration of the mutual and dependent stipulations herein contained, it is agreed by and between the parties hereto as follows:

6. Pursuant to and by virtue of continued diversions under and in accordance with the above-mentioned decree, the Company claims \_\_\_\_\_ percent of the flow of Ashley Creek at USGS Gaging Station "Sign of the Maine" corrected for diversions to Steinaker Ditch and the

Vernal City pipeline for its water users and the Company agrees with respect to its water right as follows:

a. The irrigation season is from April 1 to October 31 of each year. The irrigable land served by the Company canal is \_\_\_\_\_ acres, and the annual irrigation demand for such land as it applies to decreed water, is \_\_\_\_\_ acre-feet per acre for April and May and \_\_\_\_\_ acre-feet for the remaining months of the irrigation season, measured at the Company's headgate diversion point. During the nonirrigation season the Company's use for domestic and stock watering purposes is \_\_\_\_\_ acre-feet for each month of said nonirrigation season.

b. Under unit operation and within the limits of its decreed right in Ashley Creek, the Company must call for its water according to the following schedule:

- (1) during April, not to exceed \_\_\_\_\_ percent of its annual irrigation demand,
- (2) during May, not to exceed \_\_\_\_\_ percent of its annual irrigation demand,
- (3) during June, not to exceed \_\_\_\_\_ percent of its annual irrigation demand,
- (4) during July through October, not to exceed its decreed water rights in Ashley Creek,
- (5) during the nonirrigation season (November through March), not to exceed \_\_\_\_\_ c.f.s. of the flow of Ashley Creek.

c. It is understood that the above amounts of water have been established on the assumption that the United States will construct as part of the unit, a nonirrigation season water-saving pipe system as shown on Exhibit A attached hereto and made a part hereof so that the stockholders of the Company can run connections therefrom to their property to receive Company water during the nonirrigation season.

d. The amount of water that the Company will call for as provided above is hereinafter called Company water and the Company hereby quitclaims to the United States its right to amounts of water in excess thereof and agrees that such excess water will become part of the unit water supply, and further agrees to execute any appropriate conveyance or assignment to the United States of its water rights representing such excess water.

7. The Company hereby grants the United States the right and the United States agrees without expense to the Company, to

construct facilities as part of the Unit, to deliver Company water through the water-savings pipe system as shown on Exhibit A. The Company agrees to acquire and donate to the United States by appropriate deed of conveyance, all right-of-way needed for the water-savings pipe system. The United States agrees that during construction of the above-mentioned facilities there shall be a minimum of interference with the delivery of Company water.

8. The United States agrees, in the operation of the Unit, to deliver Company water, as established by this agreement, at the locations shown on the detail map, Exhibit B, or at such location hereafter agreed upon by the parties hereto, through facilities constructed by the United States.

9. The United States shall install a measuring device to regulate and measure the flow of water into the said Company canal and into the water-savings pipe system.

10. It is understood and agreed by the parties hereto that certain actions will have to be taken as follows:

a. The Company shall make application to the State Engineer of Utah for a change in the point of diversion of its Ashley Creek water as may be necessitated by the construction of the facilities by the United States. The United States will furnish the engineering information and detail necessary for the application.

b. The United States will make application to the State Engineer of Utah for exchange of Unit water for Company water.

c. The Company will accept delivery of Unit water in exchange for Company water that may be diverted and used on Unit lands above the Stanaker Service Canal at such times as such Unit water of equal quality and quantity can be delivered at the same locations as its Company water.

d. Upon completion of the construction of the facilities to serve the Company and after the action described in subarticles a. and b. above have been taken, the United States and the Company will advise the Water Commissioner of all actions taken and furnish him operating criteria to be followed in satisfying the Company's rights, and the exchange to be made pursuant to this contract.

11. The Company canal may need some rehabilitation and lining. The extent of such rehabilitation and lining shall be determined by the United States after consultation with the board of directors of the Company and shall be performed without expense to the Company. Should it be necessary to convey the title of the Company's canal to the United States in order to undertake the rehabilitation and lining, the Company



## WATER SUPPLY

## WATER RESOURCES

agrees to make such conveyance, provided, however, that the Company shall operate and maintain such canal at its own expense after such rehabilitation work has been performed.

12. The liability of the United States under this contract is contingent on the necessary appropriation and reservation of funds being made therefor.

13. It is understood that the covenants and agreements herein are made in anticipation of and are conditional upon, the completion of the planning, the finding of feasibility, and the eventual construction of the Unit.

14. The Company warrants that no person or selling agency has been employed or retained to solicit or secure this contract upon an agreement or understanding for a commission, percentage, brokerage, or contingent fee, excepting bona fide employees or bona fide established commercial or selling agencies maintained by the Company for the purpose of securing business. For breach or violation of this warranty the United States shall have the right to annul this contract without liability or in its discretion to require the Company to pay, in addition to the contract price or consideration, the full amount of such commission, percentage, or contingent fee.

15. No member of or Delegate to Congress or Resident Commissioner shall be admitted to any share or part of this contract or to any benefit that may arise herefrom, but this restriction shall not be construed to extend to this contract if made with a corporation or company for its general benefit.

16. The provisions of this contract shall apply to and bind the successors and assigns of the parties hereto, but no assignment or transfer of this contract, or any part thereof, or interest therein shall be valid until approved by the United States.

WATER SUPPLY

WATER RESOURCES

IN WITNESS WHEREOF, the parties hereto have caused these presents to be signed by their proper officers thereunto duly authorized, the day and year first above written.

THE UNITED STATES OF AMERICA

By \_\_\_\_\_  
Regional Director  
Bureau of Reclamation

\_\_\_\_\_  
COMPANY

(SEAL)

By \_\_\_\_\_  
President

ATTEST:

\_\_\_\_\_  
Secretary

## WATER SUPPLY

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Thus, the winter flows and high spring flows of Ashley Creek over and above beneficial use requirements would be available for Vernal unit. Studies show that the direct flow required to meet the decreed rights as modified by these contracts would result in reduced flow requirement from 682 c.f.s. to a maximum of about 325 c.f.s. as indicated in the discussion of flows available to Vernal unit.

Application No. 16387 was made by the Bureau of Reclamation for the development of the Vernal unit. This application, for the storage of 50,000 acre-feet of Ashley Creek flow in Stanaker Reservoir for use on Vernal unit land, has been approved by the State Engineer.

## WATER SUPPLY

### CHAPTER III

#### WATER REQUIREMENTS

Water requirements of the Vernal unit consist of irrigation requirements for 14,041 productive acres of presently irrigated land and municipal water supply for a present population of 5,820 which is estimated to increase by 206 percent in the next 25 years.

#### Irrigation Requirements

The detailed land classification as contained in the Project Lands Appendix lists the total area classified as 41,697 acres. Of this total 15,653 acres are class 6, 918 acres are in right-of-way and 845 acres are classified as within Vernal townsite, leaving a total area classified as irrigated of 24,281 acres. This area includes 289 acres of irrigated land under the Stanaker and Pitt Ditches above the Stanaker Reservoir normal water surface elevation.

From economic analysis Vernal unit water delivery has been limited to an acreage of class 1, 2 and 3 lands amounting to 14,731 acres. This acreage includes farmsteads, farm ditches and farm roads which are non-productive areas. These nonproductive areas were deleted from the land classification summary as shown in the following tabulation to obtain the productive acreage (14,041 acres) requiring an irrigation supply. As the farm ditch losses are accounted for in the farm efficiency, the nonproductive area was estimated to amount to 5% of the total classified area in class 1, 2, 3, and 6W. Productive acreage amounting to 95% of the total acreage for the Vernal unit is given in Table 24. Water Supply studies contained in this appendix are based on these productive acreages.

#### Vernal Unit Land

	Gross Acreage	Less R. of W.	Net Acreage	Productive acreage (95% net)
Vernal unit area	14,310	241	14,069	13,365
Vernal townsite	337	0	337	320
River Bottom area	375	0	375	356
Total	15,022	241	14,781	14,041

#### Consumptive use

Consumptive use for the Vernal unit is estimated at 1.67 acre-feet per acre. The annual consumptive use is defined as the sum of the volume of water used both by the vegetative growth of the area in transpiration or building of plant tissue and by that evaporated from adjacent soil. Consumptive water requirements of the unit area are largely dependent upon the climate, available water supply and crops grown. Although



Table 24  
Central Utah Project  
Vernal Unit  
1956 DETAILED LAND CLASSIFICATION  
PRODUCTIVE ACREAGE SUMMARY 1/

Area	Class 1	Class 2	Class 3	Vernal unit productive acreage	Class 6W	Total productive acreage
Highline Canal	149	411	430	990	751	1741
Area above Stanaker Service Canal excluding Highline Canal	2003	1669	452	4124	1052	5176
Area below Stanaker Service Canal	954	2927	4370	8251 <sup>2/</sup>	5106	13357 <sup>2/</sup>
<b>Subtotal</b>	3106	5007	5252	13365	6909	20274
River Bottom area	16	82	258	356	963	1319
<b>Total Productive Area</b>	3122	5089	5510	13721 <sup>2/</sup>	7872	21593 <sup>2/</sup>

1/ Productive acreage is 95% of net acreage. Net acreage is the gross acreage minus Vernal unit right-of-way for drains and Stanaker Service Canal.

2/ Vernal Townsite, estimated to contain 320 productive acres, is not included in this table. It is served by the Ashley Central Canal. Adding this acreage raises the Vernal unit productive acreage below Stanaker Service Canal to 8,571 acres, the total Vernal unit productive acreage to 14,041 acres, the total productive acreage below Stanaker Service Canal to 13,677 acres, and the total productive acreage to 21,913 acres.

## WATER SUPPLY

## WATER REQUIREMENTS

water requirements for irrigated projects are often computed by various methods, in the case of Ashley Valley a three-year study has been made in the area during 1948-1950. The results of this study are available in Special Report No. 8, Utah Agricultural Experiment Station, entitled "Consumptive Water Use and Requirements in the Colorado River Area of Utah". Thus, actual measurements were used as a guide in setting up water requirement figures for the Vernal unit.

The reported values in Special Report No. 8 are somewhat higher for the major crops grown in the area than values computed by the Blaney-Criddle, and Lowry-Johnson methods of consumptive determinations. It is therefore believed that the figures used in the report for Ashley Valley are high in regard to average water needs of some crops. Thus the consumptive use as found in the Special Report No. 8 have been reduced for alfalfa and pasture as shown in the following table:

	Consumptive use			
	Special Report No. 8		Recommended for Vernal Unit	
	Pre and Post		Pre and Post	
	Frost Free Period (Inches)	Frost Free Period (Inches)	Frost Free Period (Inches)	Frost Free Period (Inches)
Alfalfa	23	7	21	4
Pasture	21	7	19	4
Corn	17	-	19	-
Small pasture	17	-	17	1

In determining the amount of water requirements that would be supplied by precipitation consideration was given to the effectiveness of single storms, carry-over of winter precipitation in the form of soil moisture, and effective growing season precipitation. A conservative estimate for the net effective precipitation was considered as being 90 percent of the average precipitation occurring during the 10 driest growing seasons of record which is for the period 1920 to 1955. These data are shown in Column 6 of Table 25.

Consumptive water requirements, minus effective precipitation, gives the net amount of water that must be supplied to the crop from irrigation sources. The net consumptive use requirement for the Vernal unit is shown in Column 7 of Table 25.

The consumptive use in Special Report No. 8 is given for the major crops in the area. Estimated future cropping pattern for the unit is given in Columns 1 and 2, Table 25. In the cropping pattern it was assumed that some of the pasture would include mixtures of grass and legumes in various proportions. It was also assumed that small grains, etc. would include a small percentage of land planted to garden crops and certain other annual crops with seasonal water requirements somewhat similar to the small grains of wheat, oats, and barley.



Table No. 25  
Central Utah Project  
Vernal Unit

Consumptive Irrigation Water Requirement 1/

Crop	Percent of Area	Frost Free Period (Inches)	Pre & Post Frost Free Period (Inches)	Total Growing Period (Inches)	Effective Precipitation 2/ (Inches)	Net Consumptive Irrigation Requirement (Inches)
1	2	3	4	5	6	7
Alfalfa	37	21 <u>3/</u>	4 <u>5/</u>	25	3	22
Pasture	26	19 <u>4/</u>	4 <u>5/</u>	23	3	20
Corn	9	19 <u>6/</u>	-	19	2	17
Small grain, etc.	28	17	1 <u>7/</u>	18	1	17
Weighted Average	100					20

1/ Based on Table 11, page 22, Utah Agricultural Experiment Station, Special Report No. 8 "Consumptive Water Use and Requirements in the Colorado River Area of Utah".

2/ Estimated at 90 percent of lowest 10 years of growing season precipitation for the period 1920 to 1955.

3/ Adjusted from 23" to 21" to bring use in line with data from other methods.

4/ Adjusted from 21" to 19" to bring use in line with data from other methods.

5/ Adjusted from 7" to 4" to bring use in line with data from other methods.

6/ Adjusted from 17 to 19 to allow for maturity of crop for grain. Experimental data assumed corn would be harvested for silage.

7/ Added to take care of early planted winter grains that do consume water before the end of the growing season.

### Farm delivery

Farm delivery for the Vernal unit is estimated at 3.03 acre-feet per acre. Losses of irrigation water on the farm may be placed under three classes: (a) farm ditch losses, (b) surface runoff, (c) deep percolation. Table No. 26 gives the estimated farm irrigation water losses and efficiencies for the Vernal unit. Farm ditch losses are dependent upon slope, intake rate of the soils, and water holding capacities of the soils. Surface runoff is dependent upon slope and allowed soil moisture levels. Deep percolation is dependent upon intake rate of the soils, water holding capacities of the soils, root zone depths of the crops, and minimum soil moisture levels allowed. These limitations were used as a guide for estimating the efficiencies shown in Table 26. These estimates show from 15 to 20 percent of the water applied is lost by deep percolation which should be adequate to prevent any harmful salt accumulation within the root zone.

### Diversion requirements

Diversion requirements for the Vernal unit are estimated to be 3.7 acre-feet per acre. In addition to the farm losses, the conveyance losses incident to conveying the water from the point of diversion to the farm headgate is estimated to be 18% for the Vernal unit. In estimating these losses consideration has been given to operational losses, such as leaky headgates, change of turn loss, etc., and seepage losses in the canal banks. The administrative loss was estimated at 5% whereas the seepage loss which is dependent upon the soils through which the canals pass, the shapes, carrying capacities and length of canals is estimated at 13%, making a total weighted average conveyance loss of 18%. It is recognized that present conveyance losses are somewhat higher than 13%, and some canal lining work must be accomplished in order to bring the losses within this limitation. It is expected that losses within the Stanaker Service Canal would approximate those in the present canals after lining inasmuch as the distance the water must travel in the canal would be about the same whether it is supplied through present diversions or through the Stanaker Service Canal. Table 27 summarizes the estimated conveyance loss for the Vernal Unit.



Table 26  
Central Utah Project  
Vernal Unit

Estimated Farm Irrigation Water Losses and Efficiencies for Ashley Valley

U.S.B.R. Land Class	Slope	Intake Rate	TRAM <sup>1/</sup> in 4'	Usable Soil Depth	Seepage in Farm Ditch	Losses		Total	Farm Efficiency
			Root Zone			Surface Waste	Deep per- colation		
	Percent	In/hr	Inches	Inches	Percent	Percent	Percent	Percent	Percent
1 (23% of land)	0.5-3.0	0.5-1.0	4.5/	36 to 60	7	15	15	37	63
2 (41% of land)	(0 -0.5	0.25-0.5	3.75-4.5	30	7	25	15)	46	54
	( 3-7	1.0 -1.5		48	10	15	20)		
3 (36% of land)	( 7-11	0.15-0.25	3.50-3.75	24	7	30	15)	49.5	50.5
	(	1.5 -2.5		36	12	15	20)		
Weighted average									55

<sup>1/</sup> Total Readily Available Moisture

## WATER SUPPLY

## WATER REQUIREMENTS

Table 27  
Central Utah Project  
Vernal Unit

## Estimated Canal Seepage Losses in Ashley Valley

Canal	Length	Capacity	Loss	
			Per Mile	Total
	Mi	cfs	Percent	Percent
Highline	17	50	2.0	34
Ashley, Upper	17	300	0.75	13
Ashley, Central	12	250	0.75	9
Rock Point	12	80	1.0	12
Island	5	30	2.0	10
Dodds	2	5	4.0	8
Weighted average seepage loss				13
Administrative loss				5
Total conveyance loss				18

A summary of diversion requirements is as follows:

		Acre-feet per acre
Consumptive use		1.67
Farm efficiency	55%	
Farm delivery requirement		3.03
Conveyance efficiency	82%	
Diversion requirements		3.70

The estimated monthly diversion requirements are given in Table 28. This distribution is based on data obtained from the Moon Lake Project area which has a full water supply, modified to fit the expected type of crops, length of growing season, and need for early season water for the Vernal unit.

Table 28  
Estimated Monthly Diversion Requirements

	April	May	June	July	Aug.	Sept.	Oct.	Total
Percent	4.8	17.0	20.2	23.0	18.0	12.0	5.0	100.0
Acre-foot per acre	.18	.63	.75	.85	.67	.44	.18	3.70

## Return flow

The pattern of return flow under present practices is complex. Water diverted from Ashley Creek through the higher canals returns to lower canals and is rediverted, along with direct flow water. A few natural drainage ways are used sometimes as laterals and at other times as drains. Numerous small diversion dams on the natural drainage channels redivert seepage water onto the land. On the lower reaches of Ashley Creek the water supply for the River and Union canals consists entirely of return

flow, except for a short time during the high runoff season when some water spills past the diversion dams at the head of the valley.

In view of the general poor quality of present return flow water, the operation studies provided direct flow or reservoir water for all Vernal unit land, and return flow was not considered as being available for use. The River and Union canals, which are not within the Vernal unit irrigable area, would perhaps benefit from increased flow during the late irrigation season. Water now being used by these two canals is Class 3 or 4, or worse. It is not anticipated that operation of Vernal unit will have any appreciable effect on the quality of water for these two canals, but the desirability of using return flow can only be determined after Vernal unit is in operation. The sampling program will be continued to provide factual data for future use.

Past stream gaging records provide very little information regarding return flow. The gage Ashley Creek at "Sign of the Maine" measures runoff of Ashley Creek at the head of the valley, and the station Ashley Creek near Jensen is located in the lower part of the valley, three miles upstream from the mouth. However, the River Canal diverts about  $2\frac{1}{2}$  miles upstream from this gage, and the Union Canal diverts about  $\frac{1}{4}$  mile downstream from the gage. Hence, some of the return flow from the Vernal unit area is diverted to the River Canal before it reaches the gage, and part of the flow passing the gage during high water spills past the Union Canal diversion into the Green River. High flows measured near Jensen undoubtedly consist of spills past upstream diversion dams, return flow from water supplied to the land, surface runoff from the land, and water wasted directly into drainage channels. Surface runoff and waste is believed to be large during the early part of the year, as a result of the large streams diverted into the canals.

Flows at the lower station from July through October may be considered as return flow, but those during the rest of the year are not all return flow, especially during the high water period in May and June. Adequate records are not available at this time for a reasonable evaluation of present return flow. However, a program of stream flow measurement has been initiated, so that records will be available for determining present return flow at the lower diversions prior to completion of construction.

Under Vernal unit operations the re-use of return flow within the unit area may be expected to decrease from what it is under present practice, due to the provision for direct flow or reservoir water for all of the Vernal unit land, and to the general poor quality of return flow. It is estimated that usable return flow from Vernal unit land will amount to about 25% of the 3.70 acre-feet per acre irrigation-season diversion requirement, or about 12,000 acre-feet from the 14,041 acres. Based on

## WATER SUPPLY

## WATER REQUIREMENTS

the assumption that 60% of the return flow would occur during the month of application and 40% would occur the following month, return flow from unit land by months is estimated as follows:

<u>Month</u>	<u>Acre-feet</u>	<u>Month</u>	<u>Acre-feet</u>
April	350	August	2,400
May	1,450	September	1,720
June	2,270	October	940
July	2,630	November	240
Total			12,000

Return flow from Class 6W land within Vernal unit area would occur in addition to that from the unit land as discussed above. The unit operation should have no appreciable effect upon its amount or time of appearance. As at present, this return flow would come principally from early season irrigation water and would reduce rapidly after the high water season of May and June. It would appear in the same drainage channels as return flow from Vernal unit land, and would commingle with that water. No attempt has been made to evaluate return flow from Class 6W land.

### Evaporation

The evaporation rate is estimated to be the same at the Stanaker Reservoir site (elevation 5500) as used in Special Report No. 8, Agricultural Experiment Station, Utah State Agricultural College, "Consumptive Water Use and Requirements in the Colorado River Area of Utah". In the above report the consumptive use or evaporation from water surfaces were estimated as follows:

<u>Period</u>	<u>Evaporation</u> (inches)
Frost-free period	23.4
Pre & Part Frost-free period	8.5
Dormant season	7.8
Total for year	39.7 inches or 3.31 feet

The yearly total evaporation of 3.31 feet was distributed over the year in the same percentage by months as has been determined for Utah Lake near Lehi, Utah. Distribution percentage and monthly evaporation is given in the following table.



## WATER SUPPLY

## WATER REQUIREMENTS

<u>Month</u>	<u>% for Month</u>	<u>Evaporation for Month (Feet)</u>
Jan.	0.8	.027
Feb.	1.0	.033
March	4.6	.152
April	9.1	.301
May	13.4	.444
June	16.2	.536
July	17.7	.583
Aug.	15.9	.526
Sept.	11.6	.384
Oct.	6.6	.218
Nov.	2.3	.076
Dec.	0.8	.027
Total	100.0	3.31

Evaporation was applied to the reservoir area on a monthly basis in the Stanaker Reservoir operation study resulting in a mean annual loss of 2,100 acre-feet over the 27-year period of study.

Evaporation at Oaks Park Reservoir (elevation 9,200) is known to be less than that at the lower elevation. A rate of 2 feet was conservatively applied to the maximum water surface area of 400 acres. Distribution throughout the year was made as follows:

<u>Period</u>	<u>Acre-Feet</u>
Oct. 1 to April 30	100
May	100
June	200
July	200
August	100
September	100
Total	800

## Sedimentation

Reservoir sedimentation in the offstream Stanaker Reservoir is considered to be minor, largely because of the lack of stream flow within the Stanaker drainage basin and the good quality of water to be imported from the Ashley Creek. The estimate of sediment deposition in the reservoir was derived from available sampling data and by comparison with sediment production rates of other watersheds in the general vicinity.

No sediment sampling nor stream flow data is available from Stanaker draw, as the overall contribution to the reservoir water supply is considered to be insignificant. Occasional stream flow probably occurs in Stanaker Draw as a result of cloudburst or low intensity rainstorms throughout the summer and some minor snowmelt in the early spring.

## WATER SUPPLY

## WATER REQUIREMENTS

Geology of the drainage area shows Jurassic and Cretaceous rocks which form the flanks of the Uinta Mountains. They consist of fairly resistant sandstone and softer shales. Erosion of these formations resulted in ridges, sandstone and valleys in the shale. Stanaker Draw is an erosional valley weathered out of the soft Morrison and Aspen shales. Soil in the valley floor appears to be sandy silt loam, sandy loam and clay.

Based upon a unit yield of .19 acre-feet per square mile, sediment contribution to Stanaker Reservoir would be 3.6 acre-feet annually.

Two sediment samples have been obtained on Ashley Creek near the point of diversion. These showed a concentration of 320 ppm of suspended sediment on May 24, 1956, with a discharge of 797 cfs and a concentration of 50 ppm on March 3, 1956, at a discharge of 24 cfs. Diversions to Stanaker Reservoir would occur usually from November to June of each year. The water supply study shows a mean annual diversion of 28,200 acre-feet although a portion of this shows as spills which would not be diverted under actual operating conditions. Excess stream flow is often available during the spring runoff over and above the diversion canal capacity of 400 cfs.

It is assumed the concentration of about 320 ppm is representative of flows diverted during the months of May and June and the concentration of 50 ppm is representative of the flow during the remainder of the year. These figures are believed to be on the safe side.

Transported sediment would amount to about 5.6 acre-feet for May-June and about .6 acre-feet for the remainder of the diversions. Diverted sediment from Ashley Creek would total 6.2 acre-feet. This, together with the 3.6 acre-feet from within the basin, totals 9.8 acre-feet annually or about 980 acre-feet in a 100-year protection period.

Based upon a Type II sediment disposition curve approximately 30% of the sediment would be deposited in the dead storage pool of Stanaker Reservoir leaving 70% or 686 acre-feet to be accommodated in the active capacity. This amount comprises approximately 2% of the active capacity.

### Municipal Requirements

The Vernal unit will provide additional water to the existing municipal pipeline system that serves the towns of Vernal, Maeser, and Naples. The unit plan does not include a municipal supply for the rural areas. However, municipal requirements for Vernal unit were considered for the two different areas, are served by the present municipal water system, and the rural Ashley Valley area.

Table 29  
Past Municipal Water Use<sup>1/</sup>  
Vernal City, Maeser, and Naples

Unit: 1,000 gallons

Year	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sep.	Oct.	Nov.	Dec.	Total
<u>Vernal City</u>													
1953	12,241	12,247	17,592	24,516	29,533	47,955	50,832	39,968	35,209	21,581	15,868	17,320	324,862
1954	17,320	15,643	18,017	33,263	51,613	44,189	50,504	44,835	27,420	17,660	15,128	19,325	354,917
1955	19,325	17,456	18,716	31,593	44,776	53,239	53,959	43,199	41,389	30,425	17,620	17,181	388,878
<u>Maeser Area</u>													
1954							7,208	6,505	3,868	3,655	3,028	2,912	--
1955	2,911	2,629	2,640	4,429	6,460	7,568	6,069	4,824	4,669	5,440	3,125	3,010	53,774
<u>Naples Area</u>													
1953												1,376	--
1954	1,376	1,237	1,667	3,076	3,548	2,416	2,768	2,453	2,973	1,913	1,631	1,883	26,941
1955	1,883	1,703	1,672	2,840	4,017	3,853	3,907	3,124	3,704	2,720	1,576	1,533	32,532
<u>Total System</u>													
1954							60,480	53,793	34,261	23,228	19,787	24,120	--
1955	24,119	21,788	23,028	38,862	55,253	64,660	63,935	51,147	49,762	38,585	22,321	21,724	475,184

<sup>1/</sup> Meter readings plus 33-1/3% to account for losses and unmetered connections.

Present Municipal Water System Requirements

The municipal water system previously described in more detail serves Vernal City, Maeser and Naples. A survey of present uses from the system was made, in which meter readings were obtained for 1953, 1954, and 1955. Nineteen fifty-five was the only full year of complete records for all three communities.

There is no master meter to measure the total flow of water into the system below the storage tanks and overflow. Records of Provo, Utah, however, indicate that about 77% of the water passing the master meter reaches the metered connections throughout the city. A slightly lower efficiency of 75% was assumed for the Vernal, Maeser, and Naples system, and the meter readings were increased accordingly by 33-1/3% to obtain the total use by the system. Table 12 summarizes the past municipal use in acre-feet and Table 29 shows the total use in 1,000 gallons.

Population of Vernal City was estimated in December 1956, by the Vernal Chamber of Commerce, utilizing records of the Utah Power and Light Company, church records, and a school census. Populations of the Maeser and Naples areas served by the pipeline were obtained by an actual count. Population for years prior to 1956 were obtained from the United States Census. Table 30 summarizes the population of each area for past years.

Table 30  
Population for Past Years  
Vernal City, Maeser and Naples

Area	Population					
	1910	1920	1930	1940	1950	1956
Vernal City	836	1,309	1,744	2,119	2,845	4,300
Maeser				770	873	912
Naples		605	518	620	608	608
Total				3,509	4,326	5,820

Using the figures of water use and populations obtained above, the average daily use for 1955 was computed and shown in Table 31. Average daily use is summarized as follows:

Vernal City	248	gallons/person/day
Maeser	162	do
Naples	147	do
Weighted average for communities	224	do

For the purpose of estimating future requirements (Table 32), it was assumed that the average daily use would increase from 224 gallons per person in 1956 to 230 gallons per person by 1970, and to 240 gallons per person by 1980, after which there would be no further increase.



Table 31  
Past Municipal Water Use<sup>1/</sup>  
Vernal City, Maeser, and Naples

Unit: Gallons per person per day

Year	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sep.	Oct.	Nov.	Dec.	Mean	Population
<u>Vernal City - 4,300 population</u>														
1953	92	102	132	190	222	372	381	300	273	162	123	130	207	4,300
1954	130	130	135	258	387	343	379	336	213	132	117	145	226	4,300
1955	145	145	140	245	336	413	405	324	321	228	137	129	248	4,300
<u>Maeser Area - 912 Population</u>														
1954							255	230	141	129	111	103	-	912
1955	103	103	93	162	228	277	215	171	171	192	114	106	162	912
<u>Naples Area - 608 Population</u>														
1953												73	-	608
1954	73	73	88	169	188	132	147	130	163	101	89	100	121	608
1955	100	100	89	156	213	211	207	166	203	144	86	81	147	608
<u>Total System - 5,820 Population</u>														
1954							335	298	196	129	113	134	-	5,820
1955	134	134	128	222	306	370	354	283	285	214	128	120	224	5,820

<sup>1/</sup> Based on meter readings plus 33-1/3% to account for losses and unmetered connections.

A report on population trends was made by Howard C. Nielsen of the Stanford University Research Institute dated August 1955. Mr. Nielsen stated that the mountain region is expected to grow 66 percent in population between 1954 and 1975 or keeping about the same annual increase. Based on the assumption of Mr. Nielsen which did not take into consideration any unusual conditions which might increase the rate of population growth such as abundant electric energy and water as will be the result of the Colorado River storage project and participating projects construction, it was considered that the growth of Vernal and the surrounding area would continue as shown by the curves in Figure 3.

The ownership of Ashley Creek water by the municipal system is based upon a percent of the total stream flow, and it is not possible to evaluate the available municipal water in any year without first knowing the total stream flow for that year. For 1935, an average annual runoff year for Ashley Creek, the municipal portion of Ashley Creek flows would fail to supply the 1955 requirement by about 140 acre-feet. Municipal ownership in the Ashley Valley Reservoir Company was about 7% of the total stock in 1956, or approximately 480 acre-feet in Oaks Park and Long Park Reservoirs, Twin Lakes, Goose Lake, and Mirror Lake. For the 1935 average runoff year of Ashley Creek, the deficiency to municipal users, based on the population demand of 1955, could be made up from the storage in the reservoirs. However, with stream flows similar to those of 1934, municipal water available from Ashley Creek would fail to meet the 1955 requirement by about 380 acre-feet. It is very doubtful in such a year that the reservoirs would supply more than one-third of their average yield, or about 160 acre-feet of the 380 acre-feet shortage. Thus, as far as ownership of water is concerned, the dependable municipal supply is not equal to the present requirements.

Future requirement.--The determination of a demand for municipal water for future expansion is based on the following: (a) the anticipated population growth as shown on population curves in Figure 3, the basis of which has previously been explained, (b) the quantity of water that can be obtained from the natural flows of Ashley Creek under rights of the municipalities, (c) water obtainable from storage in Oaks Park Reservoir, Long Park Reservoir, Twin Lakes, Goose Lake, and Mirror Lake under rights of the municipalities would not exceed 380 acre-feet in normal runoff years and not more than 160 acre-feet in dry years such as 1934 and 1955.

Municipal requirements projected to years 1960, 1970, 1980, 1990, and 2000 have been compared with the supply available from Ashley Creek under present rights during the high runoff year of 1929, the medium runoff year of 1935, and the low runoff years of 1955 and 1934. These comparisons are shown in Tables 32-35. Table 32 shows that in a high runoff year like 1929 the winter flows divertible to municipal use under present rights would be inadequate to supply the water requirements of the increased population by 1970. Table 33 shows that in a normal year



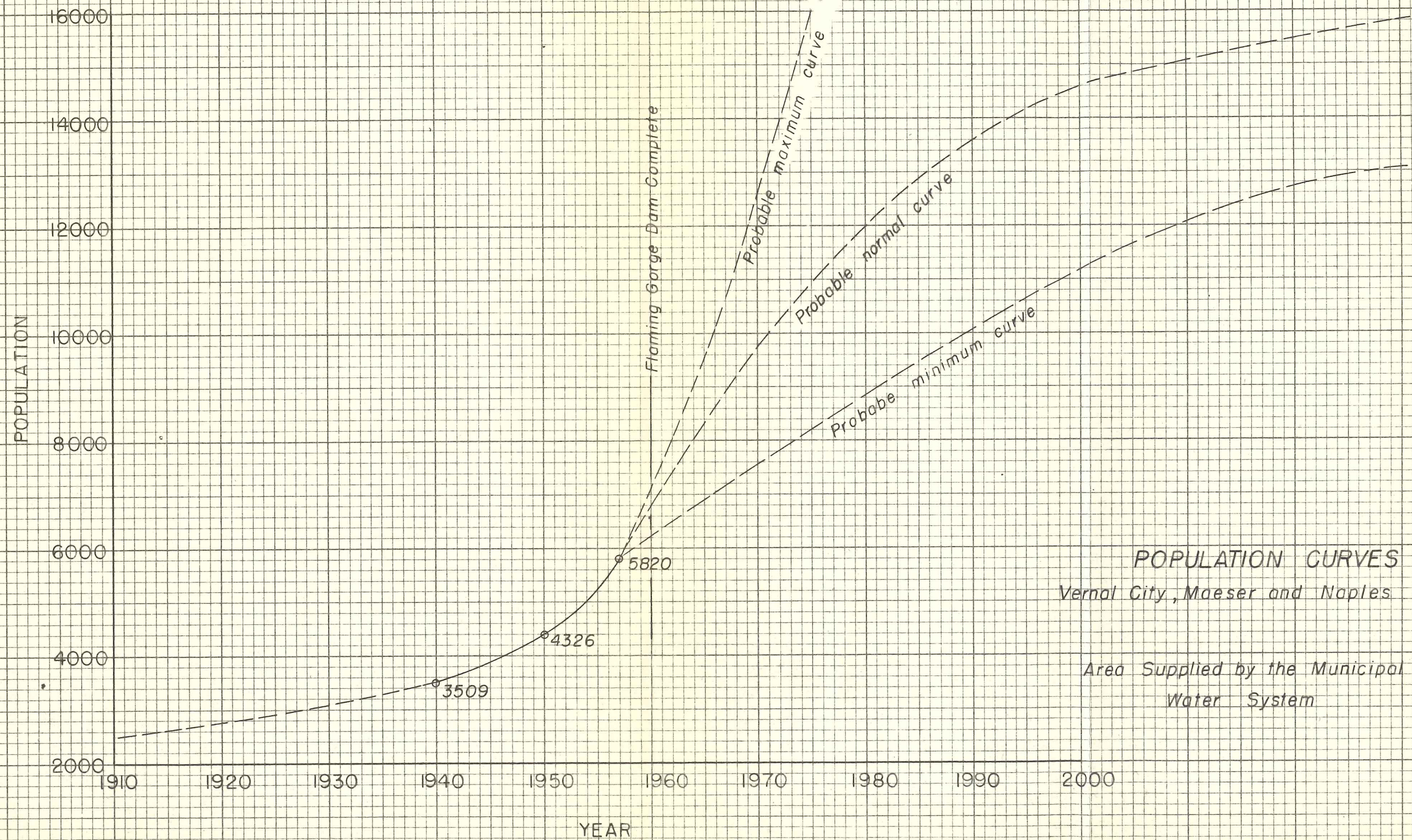


FIGURE 3



Table 32  
Central Utah Project  
Vernal Unit

Municipal Water Requirement in Addition to Ashley Creek Supply  
for a Runoff Year like 1929  
(High Runoff Year)

Year	Item	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Oct.	Nov.	Dec.	Total
1929	Streamflow owned	109	86	91	104	1662	2431	565	321	312	238	187	143	6249
1960	Demand Shortage	86	79	84	140 36	198	233	229	185	179	138	80	79	1710 36
1970	Demand Shortage	126 17	116 30	123 32	207 103	292	343	338	272	265	204	118	116	2520 182
1980	Demand Shortage	161 52	148 62	158 67	265 161	375	439	433	349 28	339 27	262 24	152	149 6	3230 427
1990	Demand Shortage	183 74	168 82	179 88	300 196	425	498	491	395 74	384 72	297 59	172	168	3660 645
2000	Demand Shortage	196 87	181 95	193 102	322 218	456	534	527	424 103	413 101	318 80	185	181 38	3930 824
	Demand %	5.0	4.6	4.9	8.2	11.6	13.6	13.4	10.8	10.5	8.1	4.7	4.6	100.0



Table 33  
Municipal water requirement in addition to Ashley Creek supply  
for a runoff year like 1955  
(Medium runoff year)

Year	Item	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Oct.	Nov.	Dec.	Total
1935	Owned	83	74	83	78	577	2,066	312	164	92	86	83	74	3,772
1960	Demand	86	79	84	140	198	233	229	185	179	138	80	79	1,710
	Shortage	3	5	1	62				21	87	52		5	236
1970	Demand	126	116	123	207	292	343	338	272	265	204	118	116	2,520
	Shortage	43	42	40	129			26	108	173	118	35	42	756
1980	Demand	161	148	158	265	375	439	433	349	339	262	152	149	3,230
	Shortage	78	74	75	187			121	185	247	176	69	75	1,287
1990	Demand	183	168	179	300	425	498	491	395	384	297	172	168	3,660
	Shortage	100	94	96	222			179	231	292	211	89	94	1,608
2000	Demand	196	181	193	322	456	534	527	424	413	318	185	181	3,930
	Shortage	113	107	110	244			215	260	321	232	102	107	1,811

Table 34  
 Central Utah Project  
 Municipal Water Requirement in Addition to Ashley Creek Supply  
 for a Runoff Year Like 1955

Year	Item	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Oct.	Nov.	Dec.	Total
1955	Streamflow owned	69	55	59	67	758	476	143	163	105	104	76	77	2152
1960	Demand	86	79	84	140	198	233	229	185	179	138	80	79	1710
	Shortage	17	24	25	73			86	22	74	34	4	2	361
1970	Demand	126	116	123	207	292	343	338	272	265	204	118	116	2520
	Shortage	57	61	64	140			195	109	160	100	42	39	967
1980	Demand	161	148	158	265	375	439	433	349	339	262	152	149	3230
	Shortage	92	93	99	198			290	186	234	158	76	72	1498
1990	Demand	183	168	179	300	425	498	491	395	384	297	172	168	3660
	Shortage	114	113	120	233		22	348	232	279	193	96	91	1741
2000	Demand	196	181	193	322	456	534	527	424	413	318	185	181	3930
	Shortage	127	126	134	255		58	384	261	308	214	109	104	2080

Table 35  
Central Utah Project  
Vernal Unit  
Municipal Water Requirement in Addition to Ashley Creek Supply  
for a Runoff Year like 1934  
(Low Runoff Year)

Year	Item	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Oct.	Nov.	Dec.	Total
1934	Streamflow owned	69	61	65	152	312	86	69	113	100	95	78	74	1274
1960	Demand	86	79	84	140	190	233	229	185	179	138	80	79	1710
	Shortage	17	18	19			147	160	72	79	43	2	5	562
1970	Demand	126	116	123	207	292	343	338	272	265	204	118	116	2520
	Shortage	57	55	58	55		257	269	159	165	109	40	42	1266
1980	Demand	161	148	158	265	375	439	430	349	339	262	152	149	3230
	Shortage	92	87	93	113	63	353	361	236	239	167	74	75	1953
1990	Demand	183	168	179	300	425	498	491	395	384	297	172	168	3660
	Shortage	114	107	114	148	113	412	422	282	284	202	94	94	2386
2000	Demand	196	181	193	322	456	534	527	424	413	318	185	181	3930
	Shortage	127	120	128	170	144	448	458	311	313	233	107	107	2666

Table 36  
Annual Summary  
Municipal Water Requirement in Addition to Present Ashley Creek Supply  
for Vernal City, Maeser and Naples Areas

				Unit: Acre-feet				
Year	Population	Total Requirement <sup>1/</sup>		Requirement in Addition to Supply Available <sup>2/</sup> from Ashley Creek with 1956 Water Rights for a Runoff Year Similar to:				
		Gallons per person per day	Acre-feet	1929 <sup>3/</sup>	1934 <sup>4/</sup>	1935 <sup>5/</sup>	1954 <sup>6/</sup>	1955 <sup>7/</sup>
1956	5,820	224	1460	20	380	140	110	180
1960	6,800	225	1710	40	560	240	250	360
1970	9,800	230	2520	180	1270	760	850	970
1980	12,000	240	3230	430	1950	1290	1480	1500
1990	13,600	240	3660	650	2390	1610	1860	1740
2000	14,600	240	3930	820	2670	1810	2090	2080

<sup>1/</sup> Includes present (1956) requirement.

<sup>2/</sup> Determined from a monthly study. This supply does not include storage right of Ashley Valley Reservoir Company totalling 480 acre-feet.

<sup>3/</sup> 1929 was the year of highest annual runoff.

<sup>4/</sup> 1934 was the year of lowest annual runoff.

<sup>5/</sup> 1935 was the year of about average annual runoff. (1925-1955).

<sup>6/</sup> 1954 runoff was about 63% of average annual runoff (1925-1955).

<sup>7/</sup> 1955 runoff was about 57% of average annual runoff (1925-1955).



## WATER SUPPLY

## WATER REQUIREMENTS

like 1935 winter and summer flows divertible to municipal use would be inadequate by 1960 and that by 1980 an additional 1,290 acre-feet, annually, would be required to supply municipal requirements. Table 34 shows that in a low year such as 1955 (57 percent normal runoff) approximately 1,500 acre-feet of additional water above present rights would be required to supply the municipal demand that would occur by 1980. In a low water year such as 1934 (34 percent of normal runoff), Table 35, the demand for additional water by 1980 would be about 2,000 acre-feet.

Allowing for 160 acre-feet of storage water available for municipal use from Oaks Park Reservoir, etc., the net requirement for additional water by 1980 in dry years such as 1955 and 1934 would be about 1,340 acre-feet and 1840 acre-feet, respectively. Of these amounts about 870 acre-feet in 1955 and 1,420 acre-feet in 1934 would be diverted during the irrigation season and would be exchanged for by releases of storage water from Stanaker Reservoir.

Table 36 shows an annual summary of the comparisons described above. On the basis of these studies and separate studies by the municipalities, resolutions have been passed by the city authorities indicating a desire and a willingness to contract with the United States for 1,500 acre-feet of unit water. Copies of the resolutions follow:

## MUNICIPAL WATER RESOLUTION OF VERNAL

WHEREAS; the anticipated growth of Vernal and its surrounding area in Ashley Valley within the next twenty-five years will render the present water supply for the area inadequate; and,

WHEREAS; according to the best estimates available, it will be necessary for the present municipal and culinary systems serving the present area and the projected growth for the future years therein to be extended; and approximately fifteen hundred acre feet per annum additional water acquired to adequately serve the area; and,

WHEREAS; additional water for municipal culinary use would be made available by the construction of Vernal Unit of the Central Utah Project by the U. S. Bureau of Reclamation.

NOW, THEREFORE, BE IT RESOLVED:

1. That Vernal City recognizes the need for additional culinary water to meet the needs of its future development.
2. That Vernal City desires its future water needs to be considered by the U. S. Bureau of Reclamation in its planning of the Vernal Unit, of the Central Utah Project.
3. That Vernal City is desirous and will be willing to contract with the proper body for the procurement of the additional needed water for its future growth and development, provided it is economically feasible.

Adopted at a regular meeting of the Vernal City Council this 20th day of February, 1957.

ATTEST:

/s/ Dixie B. Hacking

/s/ Ralph Siddoway  
MAYOR

## MUNICIPAL WATER RESOLUTION OF MAESER

WHEREAS; since the growth of the Vernal Area of the Ashley Valley within the next twenty-five years will render the present water supply for the area inadequate and incapable of supplying the needs of the future growth; and,

WHEREAS; according to the best estimates available, it will be necessary for the present municipal and culinary systems serving the present area and the projected growth for the future years therein to be extended; and additional water acquired in the amount of Fifteen Hundred Acre Feet per annum to adequately serve the area; and,

WHEREAS; since Maeser Town is presently participating in and is being served by the present existing municipal culinary water system. And its future expansion thereof must be considered prior to the building of the Vernal Unit of the Central Utah Project by the U. S. Bureau of Reclamation.

NOW, THEREFORE, BE IT RESOLVED:

1. That Maeser Town recognizes the need for water for culinary use for its future development to exist.

2. That Maeser Town desires its future water needs to be considered by the U. S. Bureau of Reclamation in its planning of the Vernal Unit, of the Central Utah Project.

3. That Maeser Town is desirous and will be willing to contract with the proper body for the procurement of the additional needed water for its future growth and development, provided it is economically feasible and acceptable to the Maeser Town.

Adopted in council assembled this 14th day of February, 1957.

MAESER TOWN BOARD

By: /s/ J. Fenon Hacking  
President

## MUNICIPAL WATER RESOLUTION OF NAPLES

WHEREAS; since the growth of the Vernal Area of the Ashley Valley within the next twenty-five years will render the present water supply for area inadequate and incapable of supplying the needs of the future growth; and,

WHEREAS; according to the best estimates available, it will be necessary for the present municipal and culinary systems serving the present area and the projected growth for the future years therein to be extended; and additional water acquired in the amount of Fifteen Hundred Acre Feet per annum to adequately serve the area; and,

WHEREAS; since the Naples Water Company is presently participating in and is being served by the present existing municipal culinary water system. And its future expansion thereof must be considered prior to the building of the Vernal Unit of the Central Utah Project by the U. S. Bureau of Reclamation.

NOW, THEREFORE, BE IT RESOLVED;

1. That the Naples Water Company recognizes the need for water for culinary use for its future development to exist.

2. That the Naples Water Company desires its future water needs to be considered by the U. S. Bureau of Reclamation in its planning of the Vernal Unit, of the Central Utah Project.

3. That the Naples Water Company is desirous and will be willing to contract with the proper body for the procurement of the additional needed water for its future growth and development, provided it is economically feasible and acceptable to the Naples Water Company.

Adopted at the regular monthly meeting of the Directors of the Naples Water Company held this 18th day of February, 1957 at the home of Charles E. Olsen.

Attest:

NAPLES WATER COMPANY

/s/ Linus V. Openshaw  
Secretary

By: /s/ Lloyd T. Pope  
President



## WATER SUPPLY

## WATER REQUIREMENTS

### Requirements for Ashley Valley Rural Areas

Rural residents living outside the three areas now supplied by the municipal water either haul culinary water to their homes or take it from irrigation ditches. Neither method provides an adequate, convenient, or sanitary water supply for culinary use.

Domestic water requirements were determined for the population as it now exists, assuming that this area will remain agricultural and that the size of farms will increase with a resultant decrease in farm population. A survey indicates that 300 families live in the Ashley Valley area. Assuming 4.22 persons per family (U.S. Census Bureau), the total population would be 1,270 persons. The Maeser town uses about 162 gallons of water per person per day and is fairly representative of the Ashley Valley rural area. Allowing a slight future increase in use, it was assumed that 175 gallons of water per person per day would be a reasonable use in the Ashley Valley area. With this figure the domestic requirement would amount to about 250 acre-feet per year. The per capita water requirements in Maeser, upon which the estimate is based, include some water for domestic livestock. A greater number of animals in proportion to the number of people is found in the other rural areas, however, so that additional water will be required for stockwatering. The additional stockwatering requirement will be needed only in the nonirrigation season. During the irrigation season many of the livestock will be on summer range away from the farm, and those remaining will be supplied from irrigation ditches. Livestock requirements were based upon the expected number and rates of water use as follows:

<u>Livestock</u>	<u>Number</u>	<u>Ave. rate of water use</u>
Cattle	16,500	10 gallons per day
Sheep	25,000	3 gallons per day
Poultry	40,000	2-1/2 gal. per day per 100 birds

Total livestock requirements would average 268 acre-feet for the entire year, and would amount to 126 acre-feet for the time they were on the farm.

The monthly distribution of domestic and stockwatering requirements of the rural areas are shown in the following table.

## WATER SUPPLY

## WATER REQUIREMENTS

Rural area water requirements (acre-feet)			
Month	Domestic	Stockwatering	Total
January	13	20	33
February	11	21	32
March	12	22	34
April	21	22	43
May	29	--	29
June	34	--	34
July	34	--	34
August	27	--	27
September	26	--	26
October	20	--	20
November	12	21	33
December	11	20	31
Total	250	126	376
Total (Nov.-April)			206

Water-saving pipe system.--During the winter months a majority of the rural homes throughout Ashley Valley obtain their stock water supply from the canals. In supplying these needs, the entire winter flow of Ashley Creek is diverted into the canals. Only a small portion of the water diverted is necessary for these uses, and the rest of the water runs to waste, resulting in a high water table and waterlogged land. The water-savings pipe system would supplant this present wasteful practice by supplying the stock water requirements, and approximately 10,000 acre-feet of water would be available for diversion to Stanaker Reservoir in the nonirrigation months.

The water-savings pipe system would begin at the Maeser-Vernal City division box of the present municipal pipeline. It would be supplied from Ashley Creek spring through the present municipal pipeline, which has about a 4 c.f.s. excess capacity during the winter months. From the division box an 8-inch steel pipe would convey the water southward for about 1/2 mile, where it would branch into two lines, one a 4-inch pipe, and the other a 6-inch pipe. The 4-inch branch would run eastward about 5 miles into the Ashley area north of Vernal. The 6-inch branch would run south and southeast for about 9 miles to the Naples area. The system would be within about a mile of all houses in the area served. Individual farmers would be required to provide their own water lines connecting their farms to the branch line.

## WATER SUPPLY

### CHAPTER IV

## WATER UTILIZATION

### Plan of Operation

Water rights on Ashley Creek at the head of Ashley Valley divide the entire flow among the water users under the Stanaker, Ashley Upper, Ashley Central, Rock Point, Island, and Dodds canals, when the flow is less than 500 cfs. Users under the Highline Canal are entitled to the flows between 500 cfs and 682 cfs.

The gaging station Ashley Creek at "Sign of the Maine" measures the stream at the head of Ashley Valley. The diversions to the municipal pipe line serving the Vernal, Maeser and Naples area are made above the "Sign of the Maine" gage and therefore are not reflected in the recorded flows at the gage. There is a small quantity of water wasted from the municipal pipe system into the irrigation system near Vernal. This waste water has not been accounted for in the operation studies because of the irregular nature of the flows and the minor volumes of water involved. Stanaker Ditch is upstream from the gaging station and diversions to this ditch do not pass the gaging station. Stanaker Ditch was therefore eliminated from the unit operation studies. It can be assumed, then, that water measured at the gaging station belongs to the other six canals.

Unit operations would be in accordance with the water rights stated briefly above. Ashley Creek flows would be used first to meet irrigation demands of presently irrigated land (Class 1, 2, 3 and 6W). Flows in excess of the irrigation demand would be diverted to Stanaker Reservoir and stored for use later when the stream flow is less than the irrigation demand. Storage water would be released to supplement stream flow on unit land (Class 1, 2 and 3) and to replace direct flow water diverted into the City pipe line that normally would be required to supply downstream prior rights.

Irrigation demand would be limited to a beneficial use of 4.0 acre-feet per acre each year for the purpose of determining surplus flows available for storage, and to 3.7 acre-feet per acre each year for the purpose of determining unit demands for storage water. In the operation study water was delivered each month of the year in accordance with the following percentages:

April	4.8	August	18.0
May	17.0	September	12.0
June	20.2	October	5.0
July	23.0	Total	100.0%

Water would be delivered from Ashley Creek to Stanaker Reservoir through the Stanaker Feeder Canal, with a capacity of 400 cfs. Water

from the reservoir would be delivered to some lands under the present canals, except Highline Canal, through the Stanaker Service Canal. Only about 63% of the unit land can be reached directly by water from the reservoir. The remaining 37% is located above the Service Canal in elevation and must be irrigated by exchange. Direct flows of Ashley Creek that are owned by users below the Service Canal would be delivered to land above the Service Canal, and storage water would be released from Stanaker Reservoir to replace the direct flows thus used.

A similar exchange would be made with water from the Oaks Park Reservoir on Brush Creek. Stockholders in the company which owns Oaks Park are located both above and below the Stanaker Service Canal. To make most efficient use of the available water supply, the Oaks Park water would be delivered to land above the Service Canal, and an exchange release made from Stanaker Reservoir.

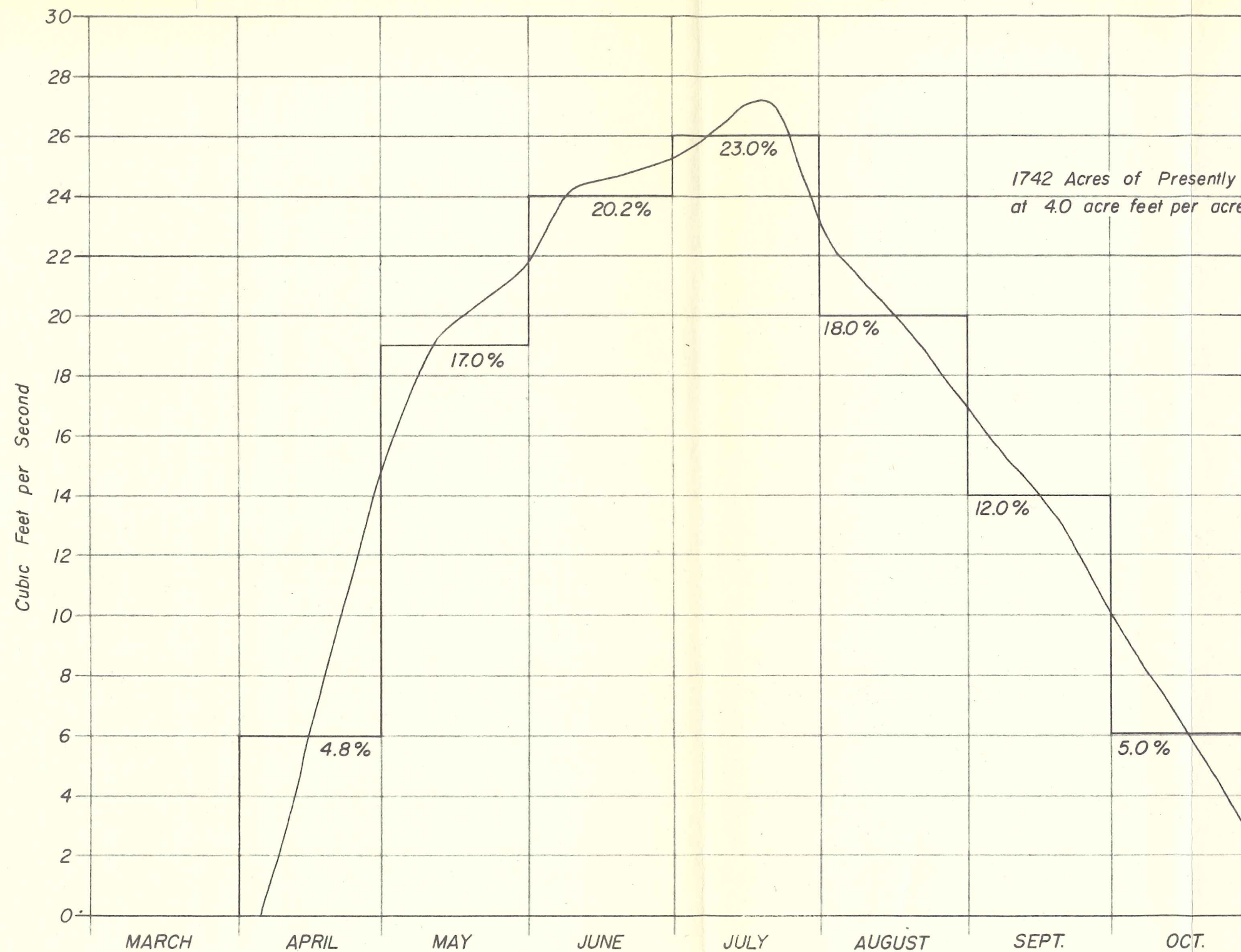
#### Ashley Creek Water Available for Vernal Unit

This is the flow of Ashley Creek at "Sign of the Maine" (not including Oaks Park Reservoir water) which is in excess of the average irrigation demand for presently irrigated land under the Highline, Ashley Upper, Ashley Central, Rock Point, Island and Dodds canals, limited during the high runoff period to a beneficial use rate of 4.0 acre-feet per acre per year, allowing 4.8% of this amount to be diverted in April, 17.0% in May, and 20.2% in June provided the water is available. The available water includes nonirrigation season runoff of Ashley Creek, which would be made available by supplying nonirrigation season stockwater needs through the proposed water-savings pipe system. Deliveries from the pipe system would be small in comparison to present deliveries accomplished by the wasteful practices of diverting the entire flow of Ashley Creek into the canals throughout the nonirrigation season. Water available to Vernal unit is assured by contracts with the major canal companies restricting their use of water to the 4 acre-feet per acre annual rate during the flood season.

For determining the water available for the unit, the Vernal unit acreage was considered in two areas: land under the Highline Canal, and land under the other 5 canals combined. For each area the average irrigation demand in cubic feet per second was determined on a daily basis for the irrigation season. This was accomplished by drawing in the monthly irrigation demand in block form in accordance with the monthly percentages given above, then sketching in a daily demand curve as shown on the drawing for the Highline Canal in Figure 4. The curve was varied until the sum of the daily demands equalled the total demand for the month.

Figure 5 illustrates the method of determining the flow in excess of the average irrigation demand. Beginning in April, when the irrigation



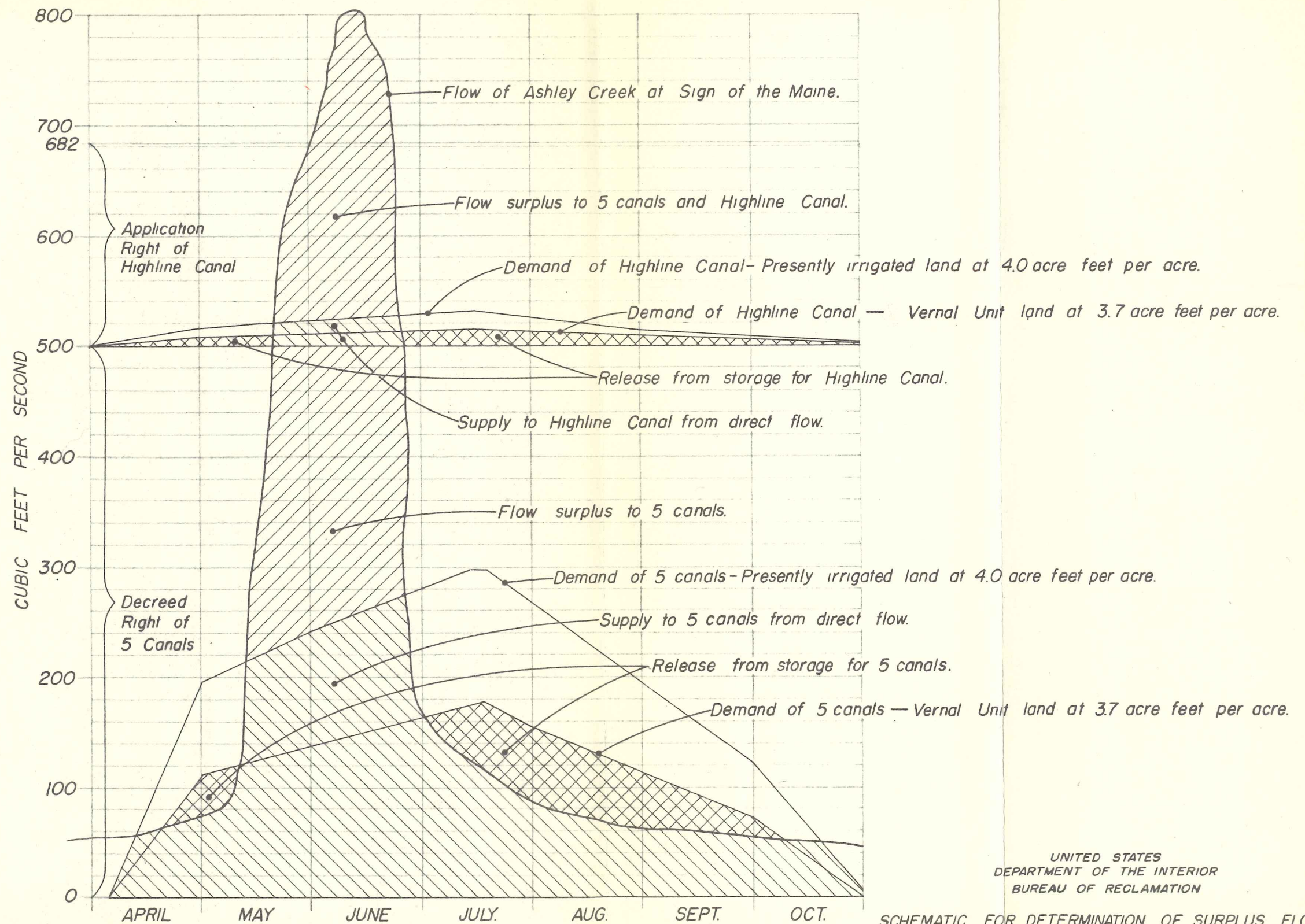


UNITED STATES  
DEPARTMENT OF THE INTERIOR  
BUREAU OF RECLAMATION

AVERAGE IRRIGATION DEMAND  
HIGHLINE CANAL

DECEMBER 14, 1956





UNITED STATES  
DEPARTMENT OF THE INTERIOR  
BUREAU OF RECLAMATION

SCHEMATIC FOR DETERMINATION OF SURPLUS FLOWS  
ASHLEY CREEK AT SIGN OF THE MAINE  
NOVEMBER 15, 1956

season normally begins, irrigators under the 5 lower canals would take all the flow of Ashley Creek until the flow exceeded the 4.0 acre-foot per acre demand line for the presently irrigated land. The flow above that demand line and below 500 cfs would be available for diversion to Stanaker Reservoir under Vernal unit operation. When the stream flow reaches 500 cfs, the rights of Highline Canal users become effective, and they would be entitled to the flow between 500 cfs and their 4.0 acre-foot per acre demand line for the presently irrigated land. Flow exceeding that demand line would be available for diversion by Vernal Unit, along with the excess flow available from the 5 lower canals. When Ashley Creek flows during the irrigation season were less than the 3.7 acre-feet per acre demand of the Vernal unit land in each area, reservoir water would be released to supply the demand up to the 3.7 acre-foot demand line. Table 37 illustrates further the determination of Ashley Creek flow available for Vernal unit for the years 1952, 1953, and 1954. Water available for Vernal unit from 1925-1956 is summarized in Table 38.

All of the water available to Vernal unit would not be diverted to Stanaker Reservoir. Flows in excess of the Stanaker Feeder canal capacity would be by-passed at the Fort Thornburgh diversion dam, and future municipal diversions would be made at Ashley Springs above the diversion point. A portion of these municipal diversions would be flow available to Vernal unit, and the remainder would be made at times when no water was available for Vernal unit, and would be replaced from Stanaker Reservoir.

#### Vernal Unit Water Divertible

That portion of the Ashley Creek flow available for Vernal unit, which could be diverted to Stanaker Reservoir by Stanaker Feeder Canal in accordance with the contracts with the canal companies has been termed the divertible flow. It was determined for the high water period utilizing mean daily flows.

Diurnal fluctuations of stream flow would cause spilling at the diversion dam when the mean daily flow was the same, or nearly the same, as the canal capacity. To account for such spills, the divertible flow was determined for the 400 c.f.s. Feeder Canal in accordance with the following schedule:

<u>Mean Daily Flow</u>		<u>Divertible Flow</u>
<u>From</u>	<u>To</u>	
0	300	Mean daily flow
301	500	75% of mean daily flow
501	up	400 c.f.s.

Table 39 summarizes the divertible flow of Ashley Creek at "Sign of the Maine".



Table 37  
DETERMINATION OF ASHLEY CREEK FLOW  
Available to Vernal Unit  
1952, 1953, and 1954

April 1952

Day	Ashley Creek Sign of the Maine	Five Canals			Flow Above 500 cfs	Highline Canal			Total Avail- able to Vernal Unit
		Demand	Divert to Five Canals	Available to Vernal Unit (Below 500 cfs)		Demand	Divert to High- line Canal	Avail- able to Vernal Unit	
	1	2	3	4	5	6	7	8	9
1	34	0		34		0			34
2	33	0		33		0			33
3	34	0		34		0			34
4	40	0		40		0			40
5	47	0		47		0			47
6	55	2	2	53		0			53
7	57	6	6	51		1			51
8	56	10	10	46		1			46
9	48	13	13	35		2			35
10	43	18	18	25		2			25
11	48	23	23	25		3			25
12	44	27	27	17		3			17
13	43	31	31	12		4			12
14	43	35	35	8		5			8
15	47	40	40	7		5			7
16	40	46	40			6			
17	38	50	38			6			
18	41	53	41			7			
19	44	58	44			7			
20	53	75	53			8			
21	60	77	60			8			
22	55	90	55			9			
23	53	101	53			9			
24	55	113	55			10			
25	65	136	65			11			
26	87	148	87			11			
27	151	161	151			12			
28	194	172	172	22		12			22
29	214	186	186	28		13			28
30	244	200	200	44		14			44
Total	2066	1871	1505	561		169			561
A. F.	3932		2865				0		1068
1000 AF	3.9		2.9				0		1.1

Explanation:

- Col. 3 Equal to or less than Col. 2.
- Col. 4 Col. 1 minus Col. 3 when Col. 1 is 500 cfs or less;  
500 cfs minus Col. 3 when Col. 1 is greater than 500 cfs.
- Col. 5 Col. 1 minus 500 cfs.
- Col. 7 Equal to or less than Col. 6.
- Col. 8 Col. 5 minus Col. 7.
- Col. 9 Col. 4 plus Col. 8.



Table 37  
DETERMINATION OF ASHLEY CREEK FLOW  
Available to Vernal Unit  
1952, 1953, and 1954

May 1952

Day	Ashley Creek Sign of the Maine	Five Canals			Flow Above 500 cfs	Highline Canal			Total Avail- able to Vernal Unit
		Demand	Divert to Five Canals	Available to Vernal Unit (Below 500 cfs)		Demand	Divert to High- line Canal	Avail- able to Vernal Unit	
	1	2	3	4	5	6	7	8	9
1	292	200	200	92		15			92
2	370	200	200	170		15			170
3	611	201	201	299	111	16	16	95	394
4	889	201	201	299	389	16	16	373	672
5	1070	201	201	299	570	17	17	553	852
6	1230	201	201	299	730	17	17	713	1012
7	1230	201	201	299	730	18	18	712	1011
8	1140	201	201	299	640	18	18	622	921
9	940	201	201	299	440	18	18	422	721
10	730	201	201	299	230	19	19	211	510
11	647	202	202	298	147	19	19	128	426
12	714	203	203	297	214	19	19	195	492
13	947	204	204	296	447	19	19	428	724
14	1090	207	207	293	590	20	20	570	863
15	1040	208	208	292	540	20	20	520	812
16	763	210	210	290	263	20	20	243	533
17	590	212	212	288	90	20	20	70	358
18	500	216	216	284		20			284
19	479	218	218	261		20			261
20	558	220	220	280	58	20	20	38	318
21	558	223	223	277	58	20	20	38	315
22	496	224	224	272		21			272
23	450	225	225	225		21			225
24	467	226	226	241		21			241
25	548	227	227	273	48	21	21	27	300
26	642	228	228	272	142	21	21	121	393
27	657	229	229	271	157	21	21	136	407
28	746	230	230	270	246	21	21	225	495
29	953	232	232	268	453	21	21	432	700
30	1050	235	235	265	550	21	21	529	794
31	1080	238	238	262	580	22	22	558	820
Total	23477	6625	6625	8429		597	464	7959	16388
A. F.	44688		12610				883		31194
1000 AF	44.7		12.6				0.9		31.2

Explanation:

- Col. 3 Equal to or less than Col. 2.
- Col. 4 Col. 1 minus Col. 3 when Col. 1 is 500 cfs or less;  
500 cfs minus Col. 3 when Col. 1 is greater than 500 cfs.
- Col. 5 Col. 1 minus 500 cfs.
- Col. 7 Equal to or less than Col. 6.
- Col. 8 Col. 5 minus Col. 7.
- Col. 9 Col. 4 plus Col. 8.

Table 37  
DETERMINATION OF ASHLEY CREEK FLOW  
Available to Vernal Unit  
1952, 1953, and 1954

June 1952

Day	Ashley Creek Sign of the Maine	Five Canals			Flow Above 500 cfs	Highline Canal			Total Avail- able to Vernal Unit
		Demand	Divert to Five Canals	Available to Vernal Unit (Below 500 cfs)		Demand	Divert to High- line Canal	Avail- able to Vernal Unit	
	1	2	3	4	5	6	7	8	9
1	1090	238	238	262	590	22	22	568	830
2	1090	242	242	258	590	22	22	568	826
3	1240	244	244	256	740	22	22	718	974
4	1260	245	245	255	760	22	22	738	993
5	1260	246	246	254	760	22	22	738	992
6	1300	248	248	252	800	23	23	777	1029
7	1210	249	249	251	710	23	23	687	938
8	1040	251	251	249	540	23	23	517	766
9	934	252	252	248	434	23	23	411	659
10	914	254	254	246	414	23	23	391	637
11	719	254	254	246	219	23	23	196	442
12	647	256	256	244	147	24	24	123	367
13	576	259	259	241	76	24	24	52	293
14	513	260	260	240	13	24	13		240
15	467	262	262	205		24			205
16	411	264	264	147		24			147
17	366	265	265	101		24			101
18	338	266	266	72		24			72
19	308	268	268	40		24			40
20	295	270	270	25		24			25
21	273	271	271	2		24			2
22	252	273	252			24			
23	238	274	238			24			
24	331	276	276	55		24			55
25	366	277	277	89		25			89
26	411	279	279	132		25			132
27	442	280	280	162		25			162
28	399	282	282	117		25			117
29	305	284	284	21		25			21
30	282	284	282			25			
Total	19277	7873	7814	4670		710	309	6484	11154
A. F.	36693		14874				588		21231
1000 AF	36.7		14.9				0.6		21.2

Explanation:

- Col. 3 Equal to or less than Col. 2.
- Col. 4 Col. 1 minus Col. 3 when Col. 1 is 500 cfs or less;  
500 cfs minus Col. 3 when Col. 1 is greater than 500 cfs.
- Col. 5 Col. 1 minus 500 cfs.
- Col. 7 Equal to or less than Col. 6.
- Col. 8 Col. 5 minus Col. 7.
- Col. 9 Col. 4 plus Col. 8.



Table 37  
DETERMINATION OF ASHLEY CREEK FLOW  
Available to Vernal Unit  
1952, 1953, and 1954

April 1953

Day	Ashley Creek Sign of the Maine	Five Canals			Flow Above 500 cfs	Highline Canal			Total Avail- able to Vernal Unit
		Demand	Divert to Five Canals	Available to Vernal Unit (Below 500 cfs)		Demand	Divert to High- line Canal	Avail- able to Vernal Unit	
	1	2	3	4	5	6	7	8	9
1	32	0		32		0			32
2	31	0		31		0			31
3	31	0		31		0			31
4	31	0		31		0			31
5	31	0		31		0			31
6	31	2	2	29		0			29
7	31	6	6	25		1			25
8	30	10	10	20		1			20
9	29	13	13	16		2			16
10	31	18	18	13		2			13
11	31	23	23	8		3			8
12	31	27	27	4		3			4
13	29	31	29			4			
14	30	35	30			5			
15	28	40	28			5			
16	28	46	28			6			
17	29	50	29			6			
18	28	53	28			7			
19	28	58	28			7			
20	28	75	28			8			
21	29	77	29			8			
22	31	90	31			9			
23	35	101	35			9			
24	40	113	40			10			
25	42	136	42			11			
26	46	148	46			11			
27	60	161	60			12			
28	72	172	72			12			
29	66	186	66			13			
30	62	200	62			14			
Total	1081	1871	810	271		169			271
A. F.	2058		1542				0		516
1000 AF	2.1		1.5				0		0.5

Explanation:

- Col. 3 Equal to or less than Col. 2.
- Col. 4 Col. 1 minus Col. 3 when Col. 1 is 500 cfs or less;  
500 cfs minus Col. 3 when Col. 1 is greater than 500 cfs.
- Col. 5 Col. 1 minus 500 cfs.
- Col. 7 Equal to or less than Col. 6.
- Col. 8 Col. 5 minus Col. 7.
- Col. 9 Col. 4 plus Col. 8.



Table 37  
DETERMINATION OF ASHLEY CREEK FLOW  
Available to Vernal Unit  
1952, 1953, and 1954

May 1953

Day	Ashley Creek Sign of the Maine	Five Canals			Flow Above 500 cfs	Highline Canal			Total Avail- able to Vernal Unit
		Demand	Divert to Five Canals	Available to Vernal Unit (Below 500 cfs)		Demand	Divert to High- line Canal	Avail- able to Vernal Unit	
	1	2	3	4	5	6	7	8	9
1	57	200	57			15			
2	51	200	51			15			
3	48	201	48			16			
4	47	201	47			16			
5	44	201	44			17			
6	46	201	46			17			
7	53	201	53			18			
8	67	201	67			18			
9	76	201	76			18			
10	69	201	69			19			
11	71	202	71			19			
12	68	203	68			19			
13	63	204	63			19			
14	62	207	62			20			
15	58	208	58			20			
16	62	210	62			20			
17	64	212	64			20			
18	84	216	84			20			
19	103	218	103			20			
20	129	220	129			20			
21	140	223	140			20			
22	185	224	185			21			
23	188	225	188			21			
24	319	226	226	93		21			93
25	256	227	227	29		21			29
26	287	228	228	59		21			59
27	256	229	229	27		21			27
28	404	230	230	174		21			174
29	392	232	232	160		21			160
30	316	235	235	81		21			81
31	382	238	238	144		22			144
Total	4447	6625	3680	767		597			767
A. F.	8465		7005				0		1460
1000 AF	8.5		7.0				0		1.5

Explanation:

- Col. 3 Equal to or less than Col. 2.
- Col. 4 Col. 1 minus Col. 3 when Col. 1 is 500 cfs or less;  
500 cfs minus Col. 3 when Col. 1 is greater than 500 cfs.
- Col. 5 Col. 1 minus 500 cfs.
- Col. 7 Equal to or less than Col. 6.
- Col. 8 Col. 5 minus Col. 7.
- Col. 9 Col. 4 plus Col. 8.



Table 37  
DETERMINATION OF ASHLEY CREEK FLOW  
Available to Vernal Unit  
1952, 1953, and 1954

June 1953

Day	Ashley Creek Sign of the Maine	Five Canals			Flow Above 500 cfs	Highline Canal		Total Avail- able to Vernal Unit
		Demand	Divert to Five Canals	Available to Vernal Unit (Below 500 cfs)		Demand	Divert to High- line Canal	
	1	2	3	4	5	6	7	8
1	392	238	238	154		22		154
2	338	242	242	96		22		96
3	332	244	244	88		22		88
4	332	245	245	87		22		87
5	374	246	246	128		22		128
6	354	248	248	106		23		106
7	436	249	249	187		23		187
8	421	251	251	170		23		170
9	399	252	252	147		23		147
10	562	254	254	308		23		308
11	719	254	254	246	219	23	23	442
12	797	256	256	244	297	24	24	517
13	940	259	259	241	440	24	24	657
14	763	260	260	240	263	24	24	479
15	590	262	262	238	90	24	24	304
16	451	264	264	187		24		187
17	388	265	265	123		24		123
18	354	266	266	88		24		88
19	322	268	268	54		24		54
20	278	270	270	8		24		8
21	241	271	241			24		
22	214	273	214			24		
23	194	274	194			24		
24	199	276	199			24		
25	192	277	192			25		
26	181	279	181			25		
27	173	280	173			25		
28	164	282	164			25		
29	152	284	152			25		
30	142	284	142			25		
Total	11394	7873	6945	3140		710	119	1190
A. F.	21688		13220				227	8242
1000 AF	21.7		13.2				0.2	8.2

Explanation:

- Col. 3 Equal to or less than Col. 2.
- Col. 4 Col. 1 minus Col. 3 when Col. 1 is 500 cfs or less;  
500 cfs minus Col. 3 when Col. 1 is greater than 500 cfs.
- Col. 5 Col. 1 minus 500 cfs.
- Col. 7 Equal to or less than Col. 6.
- Col. 8 Col. 5 minus Col. 7.
- Col. 9 Col. 4 plus Col. 8.

Table 37  
DETERMINATION OF ASHLEY CREEK FLOW  
Available to Vernal Unit  
1952, 1953, and 1954

April 1954

Day	Ashley Creek Sign of the Maine	Five Canals			Flow Above 500 cfs	Highline Canal		Total Avail- able to Vernal Unit
		Demand	Divert to Five Canals	Available to Vernal Unit (Below 500 cfs)		Demand	Divert to High- line Canal	
	1	2	3	4	5	6	7	8
1	22			22				22
2	22			22				22
3	22			22				22
4	22			22				22
5	22			22				22
6	23	2	2	21				21
7	22	6	6	16				16
8	21	10	10	11				11
9	20	13	13	7				7
10	21	18	18	3				3
11	21	23	21					
12	21	27	21					
13	22	31	22					
14	23	35	23					
15	25	40	25					
16	25	46	25					
17	26	50	26					
18	28	53	28					
19	34	58	34					
20	42	75	42					
21	49	77	49					
22	62	90	62					
23	76	101	76					
24	92	113	92					
25	123	136	123					
26	125	148	125					
27	130	161	130					
28	170	172	170					
29	170	186	170					
30	148	200	148					
Total	1629	1871	1461	168				168
A. F.	3231	3711	2898	333			0	333
1000 AF	3.2	3.1	2.9	.3			0	.3

Explanation:

- Col. 3 Equal to or less than Col. 2.
- Col. 4 Col. 1 minus Col. 3 when Col. 1 is 500 cfs or less;  
500 cfs minus Col. 3 when Col. 1 is greater than 500 cfs.
- Col. 5 Col. 1 minus 500 cfs.
- Col. 7 Equal to or less than Col. 6.
- Col. 8 Col. 5 minus Col. 7.
- Col. 9 Col. 4 plus Col. 8.



Table 37  
DETERMINATION OF ASHLEY CREEK FLOW  
Available to Vernal Unit  
1952, 1953, and 1954

May 1954

Day	Ashley Creek Sign of the Maine	Five Canals			Flow Above 500 cfs	Highline Canal			Total Avail- able to Vernal Unit
		Demand	Divert to Five Canals	Available to Vernal Unit (Below 500 cfs)		Demand	Divert to High- line Canal	Avail- able to Vernal Unit	
	1	2	3	4	5	6	7	8	9
1	127	200	127						
2	116	200	116						
3	108	201	108						
4	125	201	125						
5	177	201	177						
6	221	201	201	20					20
7	304	201	201	103					103
8	371	201	201	170					170
9	428	201	201	227					227
10	432	201	201	231					231
11	382	202	202	180					180
12	428	203	203	225					225
13	467	204	204	263					263
14	487	207	207	280					280
15	496	208	208	288					288
16	455	210	210	245					245
17	451	212	212	239		20			239
18	487	216	216	271		20			271
19	558	218	218	282	58	20	20	38	320
20	540	220	220	280	40	20	20	20	300
21	599	223	223	277	99	20	20	79	356
22	826	224	224	276	326	21	21	305	581
23	576	225	225	275	76	21	21	55	330
24	432	226	226	206		21			206
25	399	227	227	172		21			172
26	371	228	228	143					143
27	313	229	229	84					84
28	278	230	230	48					48
29	259	232	232	27					27
30	248	235	235	13					13
31	262	238	238	24					24
Total	11723	6625	6275	4849	599		102	497	5346
A. F.	23252	13140	12446	9618	1188		202	986	10604
1000 AF	23.3	13.1	12.4	9.6	1.2		.2	1.0	10.6

Explanation:

- Col. 3 Equal to or less than Col. 2.
- Col. 4 Col. 1 minus Col. 3 when Col. 1 is 500 cfs or less;  
500 cfs minus Col. 3 when Col. 1 is greater than 500 cfs.
- Col. 5 Col. 1 minus 500 cfs.
- Col. 7 Equal to or less than Col. 6.
- Col. 8 Col. 5 minus Col. 7.
- Col. 9 Col. 4 plus Col. 8.



Table 37  
DETERMINATION OF ASHLEY CREEK FLOW  
Available to Vernal Unit  
1952, 1953, and 1954

June 1954

Day	Ashley Creek Sign of the Maine	Five Canals			Flow Above 500 cfs	Highline Canal			Total Avail- able to Vernal Unit
		Demand	Divert to Five Canals	Available to Vernal Unit (Below 500 cfs)		Demand	Divert to High- line Canal	Avail- able to Vernal Unit	
	1	2	3	4	5	6	7	8	9
1	251	238	238	13					13
2	214	242	214						
3	192	244	192						
4	175	245	175						
5	170	246	170						
6	168	248	168						
7	168	249	168						
8	162	251	162						
9	152	252	152						
10	144	254	144						
11	140	254	140						
12	136	256	136						
13	129	259	129						
14	130	260	130						
15	136	262	136						
16	134	264	134						
17	129	265	129						
18	125	266	125						
19	119	268	119						
20	116	270	116						
21	116	271	116						
22	108	273	108						
23	95	274	95						
24	110	276	110						
25	123	277	123						
26	130	279	130						
27	166	280	166						
28	177	282	177						
29	144	284	144						
30	136	284	136						
Total	4395	7873	4382	13					13
A. F.	8717	15616	8692	26			0		26
1000 AF	8.7	15.6	8.7				0		0

Explanation:

- Col. 3 Equal to or less than Col. 2.
- Col. 4 Col. 1 minus Col. 3 when Col. 1 is 500 cfs or less;  
500 cfs minus Col. 3 when Col. 1 is greater than 500 cfs.
- Col. 5 Col. 1 minus 500 cfs.
- Col. 7 Equal to or less than Col. 6.
- Col. 8 Col. 5 minus Col. 7.
- Col. 9 Col. 4 plus Col. 8.

Table 38  
Summary of Ashley Creek flow available for Vernal unit  
(Unit--1,000 acre-feet)

Year	Oct.	Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Total
1930	2.0	4.3	3.3	3.0	2.3	2.4	2.7	17.2	10.0				47.2
31	1.9	3.6	3.0	2.7	2.1	2.3		1.2					16.8
32		2.1	2.1	2.0	2.0	2.0	.5	22.1	14.0				46.8
33		2.8	2.0	1.8	1.6	1.6	.4	4.4	13.1				27.7
34		1.8	1.7	1.6	1.4	1.5	.7						8.7
1935		2.2	2.1	1.9	1.7	1.9	.5	4.0	31.7				46.0
36		1.9	1.7	1.5	1.4	1.4	.4	2.3					10.6
37		2.7	1.9	1.7	1.3	1.4	.4	32.2	9.5				51.1
38		2.3	1.9	1.7	1.4	1.6	.4	14.4	26.0				49.7
39	3.5	5.1	3.5	2.7	2.1	2.4	4.1	9.8	.6				33.8
1940	1.9	3.4	2.4	1.8	1.4	1.5	1.2	8.5					22.1
41	1.0	3.3	2.4	2.0	1.6	1.6	.4	32.9	23.6				68.8
42	7.8	6.9	4.4	2.9	2.1	2.2	2.7	18.1	25.1				72.2
43		2.4	2.1	1.8	1.5	1.6	5.5	11.3	2.7				28.9
44		2.0	1.8	1.5	1.2	1.4	.5	27.9	48.5	1.0			85.8
1945		2.2	1.8	1.7	1.5	1.6	.4	7.7	8.6				25.5
46		2.4	2.0	1.7	1.4	1.4	3.3	.6					12.8
47		3.2	2.9	2.4	1.7	2.1	.7	38.0	18.2				69.2
48		2.6	2.3	1.9	1.5	1.6	.4	21.4	5.5				37.2
49		1.7	1.6	1.4	1.2	1.3	.3	16.9	24.5				48.9
1950		2.8	2.2	2.0	1.7	2.0	.9	23.4	21.3				56.3
51		2.2	2.0	1.8	1.4	1.5	.3	13.3	7.5				30.0
52		2.3	1.9	1.8	1.5	1.6	1.1	31.2	21.2				62.6
53		2.9	2.3	2.2	1.8	2.0	.5	1.5	8.2				21.4
54		2.0	2.0	1.8	1.4	1.4	.3	10.6					19.5
1955		2.1	1.8	1.6	1.2	1.4	.4	5.8					14.3
56		1.7	1.8	1.6	1.3	1.4	.3	15.4	3.7				27.2
Total	18.1	74.9	60.9	52.5	42.7	46.1	29.3	392.1	323.5	1.0			1,041.1
Mean	.7	2.8	2.3	1.9 <sup>+</sup>	1.6	1.7	1.1	14.5	11.9	0			38.5

Table 39  
Vernal unit water divertible to Stanaker Reservoir<sup>1/</sup>  
(Unit--1,000 acre-feet)

Year	Oct.	Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Total
1930	1.2	4.3	3.3	3.0	2.3	2.4	2.7	11.5	7.8				38.5
31	1.9	3.6	3.0	2.7	2.1	2.3		1.2					16.8
32		2.1	2.1	2.0	2.0	2.0	.5	12.1	12.4				35.2
33		2.8	2.0	1.8	1.6	1.6	.4	3.1	7.6				20.9
34		1.8	1.7	1.6	1.4	1.5	.7						8.7
1935		2.2	2.1	1.9	1.7	1.9	.5	3.5	15.4				29.2
36		1.9	1.7	1.5	1.4	1.4	.4	2.3					10.6
37		2.7	1.9	1.7	1.3	1.4	.4	16.5	8.6				34.5
38		2.3	1.9	1.7	1.4	1.6	.4	9.6	14.8				33.7
39	2.0	5.1	3.5	2.7	2.1	2.4	3.8	8.5	.6				30.7
1940	1.4	3.4	2.4	1.8	1.4	1.5	1.2	7.8					20.9
41	1.0	3.3	2.4	2.0	1.6	1.6	.4	16.8	17.2				46.3
42	4.5	6.9	4.4	2.9	2.1	2.2	2.7	8.2	14.0				47.9
43		2.4	2.1	1.8	1.5	1.6	5.2	9.9	2.5				27.0
44		2.0	1.8	1.5	1.2	1.4	.5	12.6	23.5	.5			45.0
1945		2.2	1.8	1.7	1.5	1.6	.4	7.7	8.0				24.9
46		2.4	2.0	1.7	1.4	1.4	3.3	.6					12.8
47		3.2	2.9	2.4	1.7	2.1	.7	21.6	15.4				50.0
48		2.6	2.3	1.9	1.5	1.6	.4	12.6	4.6				27.5
49		1.7	1.6	1.4	1.2	1.3	.3	12.6	16.4				36.5
1950		2.8	2.2	2.0	1.7	2.0	.9	11.4	14.3				37.3
51		2.2	2.0	1.8	1.4	1.5	.3	7.3	7.2				23.7
52		2.3	1.9	1.8	1.5	1.6	1.1	20.1	12.5				42.8
53		2.9	2.3	2.2	1.8	2.0	.5	1.5	7.1				20.3
54		2.0	2.0	1.8	1.4	1.4	.3	9.7					18.6
1955		2.1	1.8	1.6	1.2	1.4	.4	5.8					14.3
56		1.7	1.8	1.6	1.3	1.4	.3	13.2	3.6				24.9
Total	12.0	74.9	60.9	52.5	42.7	46.1	28.7	247.7	213.5	.5		0	779.5
Mean	.4	2.8	2.3	1.9	1.6	1.7	1.1	9.2	7.9	0			28.9

<sup>1/</sup> Flow available for Vernal unit which may be diverted to Stanaker Reservoir with a capacity of 400 cfs in Stanaker Feeder Canal. Includes surplus water diverted to future municipal use.

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This divertible flow does not correspond to the flow diverted to Stanaker Reservoir due to the future municipal diversions made at Ashley Spring upstream from the feeder canal. Part of these diversions would be made at times when flow was available for Vernal unit, and part would require exchange water from Stanaker Reservoir.

#### Vernal Unit Operation Study

The operation study for Vernal unit involved Ashley Creek, Oaks Park Reservoir located on Brush Creek, and Stanaker Reservoir. It was based on the 1930 through 1956 period of time. This period begins immediately following the excessively wet years of 1927 through 1929, because it was apparent from the runoff records that the reservoir would have spilled during those years. The excessively dry years of 1931 through 1936 are included in the study period.

To facilitate the study, the Vernal unit land was divided into three areas as follows:

##### Area 1. Land above Stanaker Service Canal excluding Highline Canal

This area is presently served by each of the 5 canals (Ashley Upper, Ashley Central, Rock Point, Island and Dodds). It cannot be reached directly by releases from Stanaker Reservoir, but would be irrigated by stream flow of Ashley Creek and by releases from Oaks Park Reservoir. It has the first water rights on Ashley Creek, along with Area 3, and these two areas were given the same priority to the use of Ashley Creek water.

##### Area 2. Land under Highline Canal

Like Area 1, the Highline Canal cannot be served directly from Stanaker Reservoir, but must depend on stream flow of Ashley Creek and Oaks Park Reservoir releases. This area has the junior right on Ashley Creek, and so was given lowest priority to Ashley Creek water. It has equal rights, however, to Oaks Park water, and users under this canal owned 21.6% of Oaks Park stock in 1956.

##### Area 3. Land below Stanaker Service Canal

This area is also presently served by each of the 5 canals. It can be reached directly by releases from Stanaker Reservoir, and comprises about 63% of the Vernal unit area. Direct flows of Ashley Creek which are owned by users in this area would be used on the two areas above Stanaker Service Canal, and reservoir water would be released in exchange for it. Part of the Oaks Park Reservoir stock is also owned by users in

## WATER SUPPLY

## WATER UTILIZATION

this area. All of the Oaks Park water would be used above the Stanaker Service Canal, and exchange water would be released from Stanaker Reservoir.

Under the operation study, the two upper areas were supplied water first from the direct flow of Ashley Creek to which they were entitled by water rights; second, from Oaks Park Reservoir; and third, by exchange from Stanaker Reservoir by using direct flow of Ashley Creek which was owned by users below Stanaker Service Canal, and which was replaced by Stanaker Reservoir releases. Vernal unit land (Class 1, 2, and 3) and Class 6W land were each allowed their proportionate share of the direct flow of Ashley Creek, but exchange water was supplied only to unit land. Shortages which occurred to both these areas were due either to there not being sufficient water physically available in Ashley Creek to meet the demand, or because there was not sufficient water in Stanaker Reservoir to replace that which was owned by the lower area and may have been used above the service canal, or to both causes.

Land in the River Bottom Area, located along Ashley Creek north of Vernal, was given 300 acre-feet of storage releases from the reservoir. This area, containing 356 productive acres, obtains its present supply from return flows and springs within the area and is in need of late season water.

### Oaks Park Reservoir

This reservoir was operated to supply water to the two areas above Stanaker Service Canal. Flow of Brush Creek at Oaks Park Reservoir, which is inflow to the reservoir, was computed for November through June for the years 1952 to 1956 inclusive, from records of reservoir content and releases. Flow for other years was estimated by correlation with Ashley Creek near Vernal (not including Oaks Park Canal). Storable flow of Brush Creek at Oaks Park Reservoir is given in Table 40. Releases were made for land above the service canal from July through September. Evaporation was considered at the rate of 2. feet per year, and a loss of ten percent was allowed for transmission losses in Oaks Park Canal and Ashley Creek. Oaks Park Reservoir active capacity was rounded to 5,700 acre-feet for purposes of the operation study.

### Stanaker Reservoir Operation

A capacity of 400 second-feet was selected for Stanaker Feeder canal, along with an active capacity of 33,200 acre-feet for Stanaker Reservoir. Releases from Stanaker Reservoir averaged 18,800 acre-feet annually, as follows:

Replacement for Oaks Park exchange	2,200	acre-feet
River Bottom area	300	" "
Vernal unit land	16,300	" "
Total release	18,800	" "

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Table No. 40  
Estimated Storable Flow of Brush Creek at Oaks Park Reservoir  
(Unit--1000 a.f.)

Year	Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	June	Nov.-June Total
1930	.6	.5	.4	.3	.3	1.0	3.7	2.4	9.2
31	.5	.4	.3	.3	.3	.4	1.3	0.8	4.3
32	.3	.3	.3	.3	.3	.3	3.7	2.7	8.2
33	.4	.3	.2	.2	.2	.2	1.3	2.2	5.0
34	.2	.2	.2	.2	.1	.5	1.0	0.4	2.8
1935	.3	.3	.2	.2	.2	.3	1.8	4.1	7.4
36	.2	.2	.2	.2	.2	.3	1.5	0.9	3.7
37	.4	.2	.2	.2	.2	.3	4.8	2.4	8.7
38	.3	.2	.2	.2	.2	.4	3.1	3.7	8.3
39	.7	.5	.4	.3	.3	1.2	2.6	1.1	7.1
1940	.5	.3	.2	.2	.2	.7	2.6	1.0	5.7
41	.5	.4	.3	.2	.3	.2	4.7	3.4	10.0
42	1.0	.6	.4	.3	.3	.9	3.5	3.9	10.9
43	.3	.2	.2	.1	.2	1.3	2.5	1.6	6.4
44	.2	.2	.2	.2	.2	.2	3.5	5.5	10.2
1945	.3	.2	.2	.1	.2	.2	2.3	2.3	5.8
46	.3	.2	.2	.2	.2	0.9	1.3	0.8	4.1
47	.4	.4	.3	.2	.2	.6	4.9	3.1	10.1
48	.4	.3	.3	.2	.2	.3	3.7	1.9	7.3
49	.2	.2	.2	.1	.1	.4	4.2	3.6	9.0
1950	.3	.3	.2	.2	.2	.9	3.9	3.5	9.5
51	.3	.3	.2	.2	.2	.2	2.3	1.7	5.4
52	.4	.3	.3	.2	.2	.6	5.7	3.6	11.3
53	.3	.3	.3	.2	.2	.3	1.4	2.5	5.5
54	.3	.2	.2	.2	.2	.6	2.7	1.0	5.4
1955	.3	.2	.2	.1	.2	.2	2.3	1.5	5.0
56	.3	.2	.2	.2	.2	.3	3.5	1.6	6.5
Total	10.2	7.9	6.7	5.5	5.8	13.7	79.8	63.2	192.8
Mean	.4	.3	.2	.2	.2	.5	3.0	2.3	7.1



## WATER SUPPLY

## WATER UTILIZATION

## Vernal Unit shortages

With the total demand averaging 51,700 acre-feet annually, Vernal unit shortages averaged 2,000 acre-feet annually, or 4% of the total demand. The maximum shortage occurred in 1934, and amounted to 23,100 acre-feet, or 45% of the total demand.

## Summary

Table 41 is a copy of the Vernal unit operation study by months for the years 1952, 1953, and 1954. An annual summary of the Vernal unit operation study is given in Table 42, and Figure 6 illustrates the behavior of Stanaker Reservoir under the operation study. The water supply of Vernal unit land under Vernal unit conditions is summarized in Table 43.

Capacity of Stanaker Service Canal

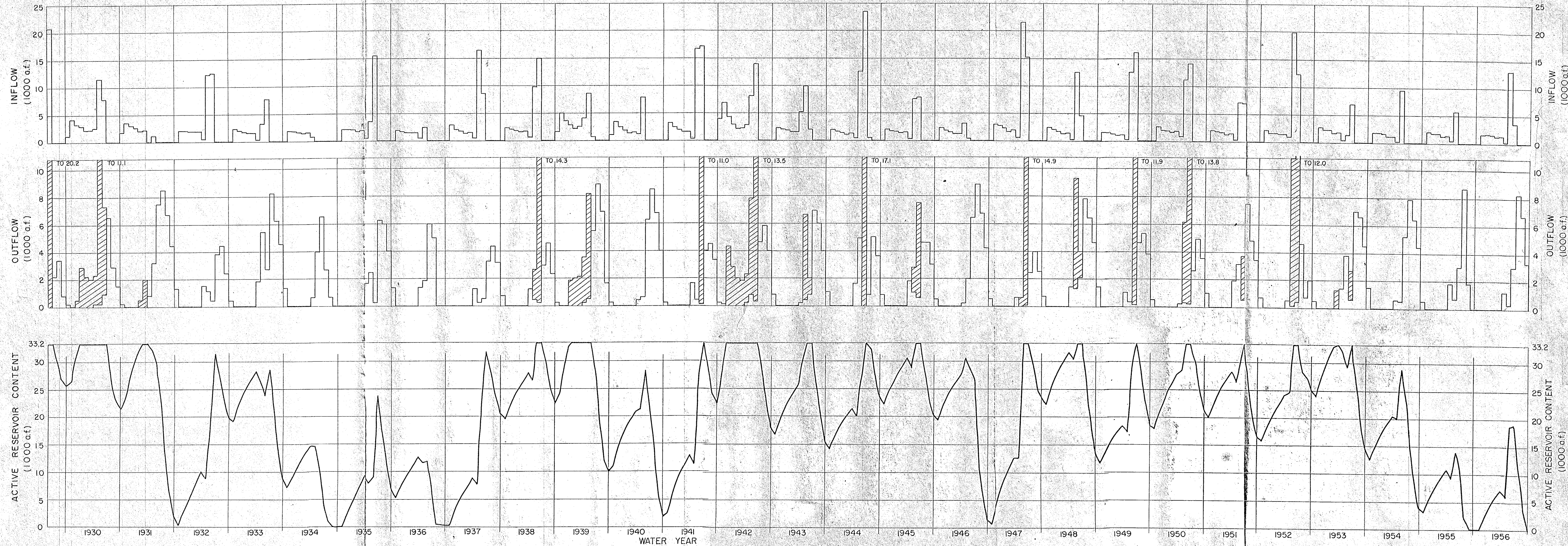
Design capacity of the Stanaker Service Canal was based upon a study of actual diversions made to canals of the Strawberry Valley, Provo River, and Moon Lake projects. Maximum mean daily flows in each canal were expressed as a decimal fraction of the mean monthly flow, for the month of maximum diversion for each year of the 1945 to 1954 period. Results of this study are tabulated below.

Project	Canal	Max. Mean Daily Flow	
		Mean Monthly Flow	
Strawberry Valley	Springville-Mapleton	1.39	
	Highline	1.39	
	East Bench	1.55	
	Lake Shore	1.55	
	Salem	1.15	
	Average		1.41
Provo River	Provo Bench	1.12	
	Murdock	1.09	
	West Union	1.22	
	Provo City	1.16	
	Average		1.15
Moon Lake	U. S. Lakefork	1.10	
	Red Cap	1.49	
	Farnsworth	1.26	
	Dry Gulch No. 1	1.27	
	Average		1.28
Total			3.84
	Average		1.28



[illegible]





EXPLANATION

Inflow

Spill Release

Active reservoir content

Note:

Active reservoir capacity - 33,200 acre feet

Feeder canal capacity - 400 cubic feet per second

UNITED STATES  
DEPARTMENT OF THE INTERIOR  
BUREAU OF RECLAMATION  
CENTRAL UTAH PROJECT  
VERNAL UNIT - UTAH

**VERNAL UNIT OPERATION STUDY**  
**STANAKER RESERVOIR HYDROGRAPHS**  
SPANISH FORK, UTAH

MAY 10, 1957







ir Operation

Active reservoir content				Shortage						Municipal Shortage		Water not Used (Bypass plus Stanaker Reservoir Spill)	Year		
Spill	Max	Min	End of year	Irriga- tion for River Bottom Area	Land above Stanaker Service Canal	Highline Canal	Land below Stanaker Service Canal	Irrigation Shortage		1000 Acre-feet	Percent			1000 Acre-feet	Percent
								2/ Total Irrigation Shortage							
78	79-a	79-b	79-c	80	81	82	83	1000 Acre-feet	Percent	84-a	84-b	85-a	85-b	86	
27.0	33.2	31.2	25.8												
2.5	33.2	1.7	21.3		1.1	1.3		2.4	5					35.4	1930
	31.2	.4	19.5											2.5	31
	28.3	8.5	8.5											11.3	32
	14.5	-1.1	-1.1			.3		.3	1					6.5	33
	23.6	-1.1	6.6	.3	9.5	2.5	10.6	23.1	45			.2	13		34
	12.5	.1	.1		.3	.2	.7	1.2	2					16.5	1935
	31.5	.1	20.2	.3	5.0	1.7	5.9	12.9	25						36
16.3	33.2	19.4	22.3		.1	.2	.2	.5	1					16.3	37
16.8	33.2	9.7	9.7											32.0	38
	28.0	1.7	1.7			.4		.4	1					19.6	39
					.3	.3		.6	1					1.0	1940
10.9	33.2	2.4	22.3											33.1	41
33.3	33.2	17.7	17.7											57.2	42
7.4	33.2	15.2	15.2											8.9	43
17.1	33.2	14.1	23.2											57.4	44
8.7	33.2	19.9	19.9											9.1	1945
	30.3	1.4	1.4			.2		.2	0						46
15.4	33.2	.9	23.2				.2	.2	0					34.3	47
9.8	33.2	13.3	13.3											19.3	48
11.8	33.2	11.8	18.1											23.9	49
19.4	33.2	17.8	21.2											38.1	1950
3.2	33.2	16.5	15.5											9.2	51
22.4	33.2	15.7	28.7											41.9	52
3.4	33.2	14.1	14.1											4.3	53
	28.7	3.7	3.7											.8	54
	13.9	-1.1	-1.1	.5	2.8	1.0	5.5	2.6	19						1955
	18.9	-1.1	0	.1	1.5	.7	.7	3.0	6					2.1	56
225.4	--	--	--	1.0	20.6	8.8	23.8	54.4	--	.2				480.7	Total
8.3	--	--	--	0	.8	.3	.9	2.0	45	0	0%			17.8	Mean

Stanaker Reservoir active capacity: 33,200 acre-feet  
Stanaker Feeder Canal capacity: 400 cubic feet per second

Table 43  
Annual water supply summary--Vernal unit lands  
(Unit--1,000 acre-feet)

Year	Streamflow						Stanaker Reservoir operation					Land above Stanaker Service Canal exclusive of land under Highline Canal <sup>5/</sup>				Land under Highline Canal <sup>8/</sup>				Land below Stanaker Service Canal <sup>9/</sup>				Total for land under major canals <sup>11/</sup>						River Bottom area		Total unit shortages				Year																																																																																																																																																																																																																																																																																																																																																																																																																																																																												
	Ashley Creek flow at "Sign of the Maine" <sup>1/</sup>	Future increase in municipal diversions	Required for prior rights <sup>2/</sup>	Surplus flow at Ft. Thornburgh Diversion Dam <sup>3/</sup>	Bypass <sup>4/</sup>	Diverted to Stanaker Reservoir	Evapo-ration	Total release	Spill	Active reservoir content		Water supply <sup>7/</sup>		Addi-tional water at rate of 4 ac.-ft. per acre <sup>7/</sup>	Water supply		Addi-tional water at rate of 4 ac.-ft. per acre <sup>7/</sup>	Water supply		Addi-tional water at rate of 4 ac.-ft. per acre <sup>7/</sup>	Water supply				Water supplied from Vernal unit <sup>12/13/</sup>	Shortage	Irrigation <sup>14/</sup>				Municipal water																																																																																																																																																																																																																																																																																																																																																																																																																																																																																	
										Maximum	End of year	1,000 acre-feet	Percent of annual demand		Addi-tional water from Vernal unit <sup>6/</sup>	1,000 acre-feet		Percent of annual demand	Addi-tional water from Vernal unit <sup>9/10/</sup>		1,000 acre-feet	Percent of annual demand	Addi-tional water from Vernal unit <sup>9/12/</sup>	1,000 acre-feet			Percent of annual demand	Addi-tional water at rate of 4 ac.-ft. per acre <sup>7/</sup>	Present supply within ideal demand (3.7 ac.-ft. per acre)	Addi-tional water from Vernal unit <sup>6/</sup>	1,000 acre-feet	Percent of annual demand	Addi-tional water at rate of 4 ac.-ft. per acre <sup>7/</sup>	Present supply within ideal demand (3.7 ac.-ft. per acre)	Addi-tional water from Vernal unit <sup>6/</sup>		1,000 acre-feet	Percent of annual demand	Addi-tional water at rate of 4 ac.-ft. per acre <sup>7/</sup>	Present supply within ideal demand (3.7 ac.-ft. per acre)	Addi-tional water from Vernal unit <sup>6/</sup>	1,000 acre-feet	Percent of annual demand	Addi-tional water 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<sup>7/</sup>	Present supply within ideal demand (3.7 ac.-ft. per acre)	Addi-tional water from Vernal unit <sup>6/</sup>

1/ Exclusive of upstream diversions to Stanaker Ditch and the municipal pipeline.

2/ Required for all presently irrigated land, with diversions limited to a rate of 4 acre-feet per acre annually by agreements made for Vernal unit operation.

3/ Flow of Ashley Creek at "Sign of the Maine" in excess of downstream rights supplied at rate of 4 acre-feet per acre annually less 1,500 acre-feet annually that will be diverted above gage to supply municipal requirements.

4/ Bypass is available flow in excess of capacity of Stanaker Feeder Canal. In actual practice bypass may be increased to reduce reservoir spill, Column 9.

5/ Demand is 15,300 acre-feet annually.

6/ Supplied under annual demand of 3.7 acre-feet per acre.

7/ Water in excess of ideal requirements (3.7 acre-feet per acre) divertible by prior rights from natural streamflow during high water periods under agreements limiting prior rights to 4 acre-feet per acre.

8/ Demand is 3,700 acre-feet annually.

9/ Demand is 32,400 acre-feet annually.

10/ Includes an average of 800 acre-feet of water released in exchange for future municipal diversion from Ashley Springs during the irrigation season. The remaining 700 acre-feet of municipal diversions would be made during the nonirrigation season when exchange would not be necessary.

11/ Demand is 51,400 acre-feet annually.

12/ Total water supplied would be: Reservoir release (Col. 28 & 32) 18,800 acre-feet  
Less municipal exchange 800 " "  
Irrigation supply 18,000 " "  
Vernal Unit municipal supply 1,500 " "  
Total 19,500 " "

13/ River Bottom area comprises 356 acres, which require 300 acre-feet of Vernal unit water for a full supply.  
14/ Total irrigation demand is 51,700 acre-feet including 51,400 for land under major canals and 300 acre-feet for land in River Bottom area.



## WATER SUPPLY

## WATER UTILIZATION

Due to its location in the Uinta Basin, the factors of the Moon Lake project were considered to be more applicable to Ashley Valley, and a factor of 1.30 was selected for the Vernal unit.

Twenty-three percent of the annual demand would be delivered during July, the month of maximum demand, or 0.85 acre-feet per acre for the month. Using the 1.30, one second-foot of flow would be required for each 55 acres of land. The initial capacity of Stanaker Service Canal would be 250 cfs for approximately 13,700 acres under the canal. However, the portion of the canal between Stanaker Dam and Ashley Creek is being designed as a spillway for the reservoir, and will have an initial capacity of 300 cfs, well above that required for irrigation. After crossing Ashley Creek, the service canal would be designed to irrigate 11,500 acres, with an initial capacity of 210 second-feet.

### New Water Supplied by Vernal Unit

New water supplied by the unit would amount to the difference between the average water supply under unit conditions and that under present conditions, limited to theoretical demand during months of high runoff. The water supply under present conditions was determined by a daily study. Diversions with the unit were obtained from the results of the Stanaker Reservoir operation study (Table 42), for unit land (Class 1, 2, and 3). To this was added the present supply for nonunit land (Class 6W) to obtain the total supply with the unit in operation. New water supplied by the unit would amount to 18,000 acre-feet per acre for irrigation and 1,500 acre-feet per year for municipal use. Table 44 summarizes the new water supplied.

Table 44  
Central Utah Project  
Vernal Unit

New water supplied by Vernal unit	
Area	Acre-feet annually
Land under Highline Canal	2,500
Land above Stanaker Service Canal	4,800
Land below Stanaker Service Canal	11,200
River Bottom area	300
Subtotal	18,800
Less Municipal exchange	800
Irrigation supply	18,000
Municipal supply	1,500
Total	19,500

## WATER SUPPLY

### CHAPTER V

#### COLORADO RIVER DEPLETIONS

##### Present Streamflow Depletions

As a result of present development in the Ashley Creek drainage basin the available water supply for Ashley Valley is depleted by evaporation from reservoirs, by domestic water consumption and by consumptive use of water resulting from irrigation applications. The presently constructed reservoirs on Ashley Creek are very small and are located high in the drainage area. The evaporation is therefore small and was neglected in the present stream depletion estimates. The present diversion for domestic use is 1,460 acre-feet annually, of which one half is estimated to be consumed. Under unit development the estimated consumptive use is estimated at 2.00 acre-feet per acre or 54% of the diversion requirements. Present practices use more of the early spring runoff applied to the land in an apparent attempt to create soil holdover storage for late season use. Thus the consumptive use would be less under present practices than with unit development. Present diversions are estimated at 50,000 acre-feet and a 45% consumptive use. Thus, present stream flow depletions would amount to about 730 acre-feet for domestic water and 22,500 acre-feet for irrigation, or a total of 23,230 acre-feet.

##### Future Streamflow Depletions

Streamflow depletions under unit development include evaporation from reservoirs, domestic water consumption, and consumptive use of water resulting from irrigation application.

##### Reservoir evaporation

Increased depletions under unit development include evaporation from Stanaker Reservoir which is estimated at 2,100 acre-feet annually as shown in column 73, Table 42. Other evaporation losses would remain essentially the same as at present.

##### Domestic use

Additional municipal and industrial water uses in the Vernal area are expected to amount to 1,500 acre-feet annually with a consumptive use of 700 acre-feet. Present winter flows that are being diverted into canals for stockwater use will, by exchange, be conveyed to some areas by the Water Savings pipe system. This will reduce the present nonirrigation season diversion for stockwater from an estimated 10,000 acre-feet to 126 acre-feet with an estimated savings in consumptive use of from 1,100 to 100 or a net savings of 1,000 acre-feet.

# WATER SUPPLY

## COLORADO RIVER DEPLETIONS

### Irrigation use

Operation studies show the average reservoir release for irrigation as 18,000 acre-feet. Under unit development the consumptive use is estimated at 54% of the diversion requirements or 10,000 acre-feet which includes the irrigation consumptive use and uses incidental to irrigation.

### Summary of increased depletion

A summary of the increased Colorado River depletion attributable to unit development is as follows:

<u>Purpose</u>	<u>Increase in depletion (acre-feet per year)</u>
Reservoir evaporation	2,100
Domestic use (Vernal, Maeser, and Naples)	700
Irrigation and uses incident to irrigation	10,000
Subtotal	12,800
Saving by Water Savings Pipeline	1,000
Total	11,800



## WATER SUPPLY

### CHAPTER VI

#### FLOOD CONTROL

##### Previous Studies

Little damage is caused by floods on Ashley Creek, according to Amos W. Hoggard, Associate Engineer of the U. S. Engineers Office who made a reconnaissance flood control survey of Uinta Basin streams in February 1943. He estimates that the annual flood damage to diversion dams does not exceed \$300 to \$400 annually. He reported that the old U. S. Highway Bridge had been damaged some, but not since the construction of the new bridge.

Mr. Hoggard's report was very preliminary in nature and was not intended as a unit report. Therefore, in connection with preparation of the Vernal unit report, Mr. F. N. Cronholm of the U. S. Engineers Office in Salt Lake City made a detailed inspection of the unit area for the purpose of making recommendations regarding further flood control studies. He considered the flood damages to be of greater magnitude than was indicated in the report by Mr. Hoggard and recommended that a more detailed study be made.

In connection with the Vernal unit investigations, the Bureau of Reclamation made the flood control study recommended by Mr. Cronholm. This study is reported below.

##### Flood Flows

Floods on Ashley Creek are confined almost entirely to those resulting from snowmelt in the high Uinta Mountains during the spring months. As temperatures increase in the spring stream flows rise rapidly and remain at a high level for about 15 days then decrease to normal summer flows. Climatic factors such as snow cover, temperature, humidity, wind, and rain affect the occurrence and duration of peak flows. Summer cloudbursts, usually on the lower mountain slopes, occasionally produce high flows but seldom do any damage because of their short duration.

##### Historical Floods

In 1900 discharge records were started on Ashley Creek near Vernal above the confluence with Dry Fork. This record continued until 1904 when it was discontinued. In 1912 and 1913 partial records were kept during the winter season, although no records of flood flows were obtained. A record was started again in 1915 which has been continuous since then except for short periods.

A gage was established below the confluence with Dry Fork at the "Sign of the Maine" in June 1939. A continuous record is available since that

time. Peak flow for the period of record occurred on July 21, 1945, with a maximum discharge of 2,650 second-feet.

The peak flood at the gage near Vernal occurred in May 1921, the maximum instantaneous flow reaching 2,051 second-feet on May 29. It was concluded from newspaper clippings and discussion with old-time residents that this was the maximum flood since 1890. A flood of considerable magnitude occurred between 1880 and 1890 which caused the Ashley Creek channel to change its location for a distance of 8 miles. Data from which estimates of the magnitude of this flood could be made are lacking. It was believed by one of the old-time residents that the old diversion for the central canal was a contributing cause of the stream changing its channel. Ashley Creek has remained in this new channel except for minor changes since then.

#### Flood Damages

Past flood damages have consisted of the complete channel changes that occurred in the 1880's and damages to canals and diversion structures, roads and bridges, land adjacent to the channel, improvements such as fences and farm buildings. Using these past damages as a basis and considering new improvements and increased values, the average annual future flood damage on Ashley Creek is estimated as follows:

	Average annual damage
Canals and diversion structures . . . . .	\$1,620
Roads and bridges . . . . .	4,080
Land adjacent to channel. . . . .	800
Shifting of channel . . . . .	2,550
Total	9,530

Amortized at 3 percent over 50 years, this damage would amount to \$245,000.

#### Flood Control Measures

To provide complete flood protection on Ashley Creek in the Vernal unit it would be necessary to provide a feeder canal of sufficient capacity to carry the maximum probable flood flow in excess of the safe carrying capacity of Ashley Creek and to have sufficient capacity in Stanaker Reservoir to hold the volume of water during the maximum flood in excess of the volume that could be safely by-passed by Ashley Creek.

In determining the safe carrying capacity of Ashley Creek, cross-sections were taken at critical sections along the channel through the flood danger area. From these the slope and the average minimum cross-sectional area were determined. From observation it was found that the

predominant size of rock in the streambed is about 2 inches in diameter. An average velocity of approximately 6 feet per second is required to move material of this size. It was therefore concluded that if the average velocity is below this amount aggradation of the channel would cease and the possibility of the stream changing courses would be eliminated. To maintain the velocity below 6 feet per second the flow should not exceed 700 second-feet. However, peaks of short duration (less than 12 hours) in excess of 700 second-feet were considered allowable. It is noted from the hydrograph of the 1921 flood at "Sign of the Maine" that all peaks above 2,700 second-feet are less than 12 hours in duration. It was therefore assumed that the channel would carry these peaks in excess of the 700 second-feet without damage. The required feeder canal capacity to carry the flow between these limits is then 2,000 second-feet. A reservoir capacity of 68,000 acre-feet would be required to hold the excess flow of the 1921 flood.

#### Regulation Costs

To provide regulation for floods in the Vernal unit plan would require increasing the capacity of the feeder canal from 400 to 2,000 second-feet and the capacity of the reservoir from 37,000 to 68,000 acre-feet. The spillway and the wasteway canal from Stanaker Reservoir would also have to be enlarged to carry an additional 1,600 second-feet. Rough comparative cost estimates showed that these additional features would cost approximately \$3,100,000 based on 1956 prices. The estimated benefits over 50 years as outlined above totaled \$245,000. Because of this great excess in costs over benefits detailed studies were not made and flood control works were excluded from the unit plan.

#### U. S. Army Corps of Engineers Report

A summary of the Corps Report which is included in the Definite Plan Report is as follows:

"Floods on Ashley Creek damage irrigation structures, roads, and bridges, and erode farmlands for about 8 miles in the Central Part of the valley. About 2,700 acres of farmland are affected. However, most of the project lands are free from any flood menace. Present flood damage has been estimated by the Bureau of Reclamation at \$9,500 annually. Results of investigations indicate that the benefits from preventing this damage would not justify either the construction of local protection works in the project area or the provision of flood control storage space in a reservoir on Ashley Creek above the Valley. The conclusion is reached, therefore, that the construction of flood control features as part of the Vernal Unit is infeasible."



Inflow Design Flood for Stanaker Dam Site

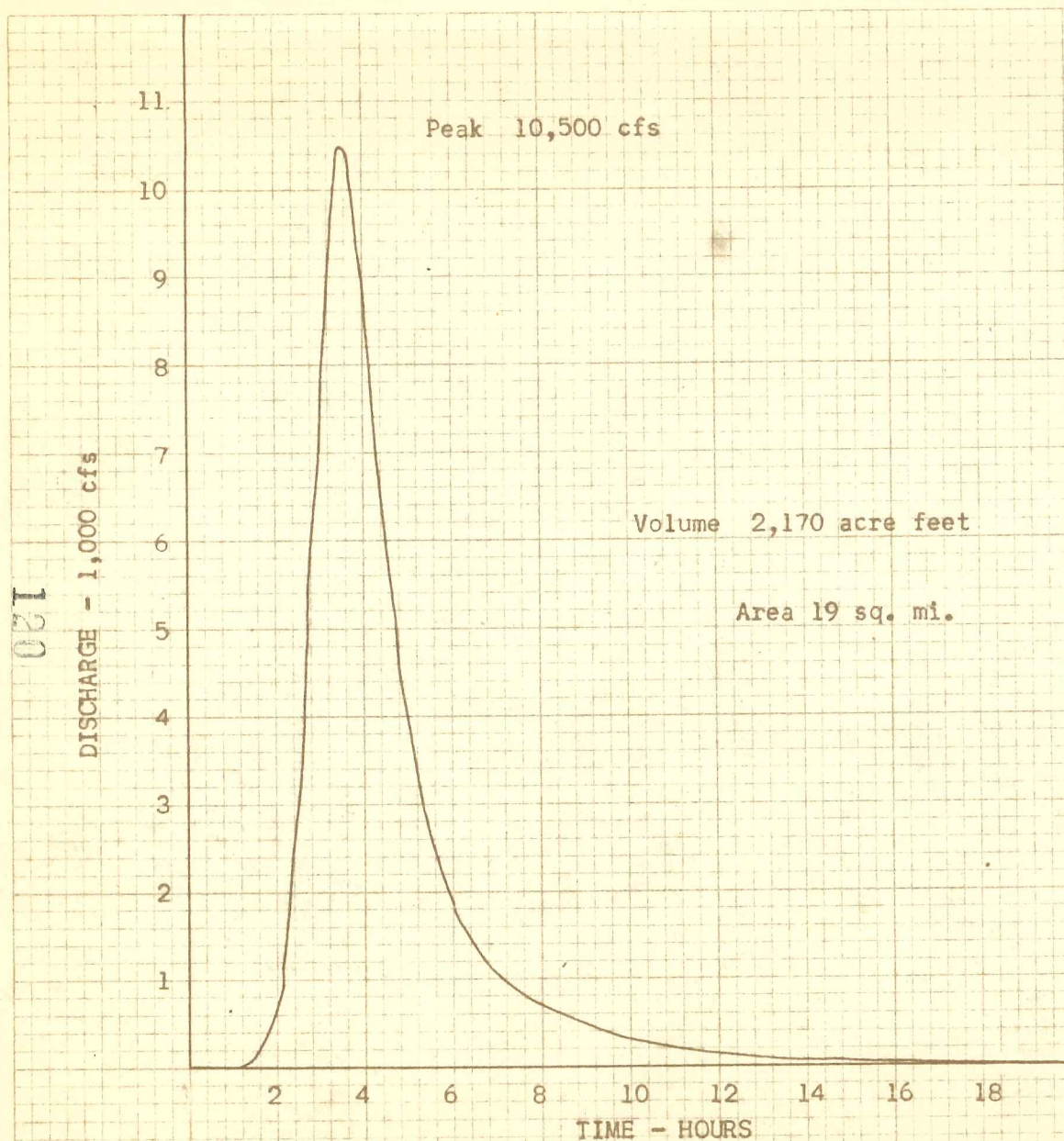
An inflow design flood study has been made on the Stanaker Draw drainage area for the Stanaker Dam site by the Denver Project Investigations Division in June 1956. The inflow design flood derived in this study has a peak of 10,500 second-feet and an 18-hour volume of 2,170 acre-feet, as shown in Figure 7.

Diversion requirement hydrographs having a recurrence interval of once in 5, 10, and 25 years, are presented graphically in Figure 8 which have the following characteristics:

<u>Frequency in years</u>	<u>Peak discharge cfs</u>	<u>Volume (12-hour) acre-feet</u>
5	400	70
10	1,300	230
25	2,250	400

Flood Frequency Study Ft. Thornburgh Diversion Dam

A flood frequency study to determine the peak flow at the Ft. Thornburgh diversion dam site was made in July 1950 and reviewed in November 1956. This study recommended a design peak of 3,400 cfs.



United States  
Department of the Interior  
Bureau of Reclamation  
DIVISION OF PROJECT INVESTIGATIONS  
Central Utah Project - Vernal Unit

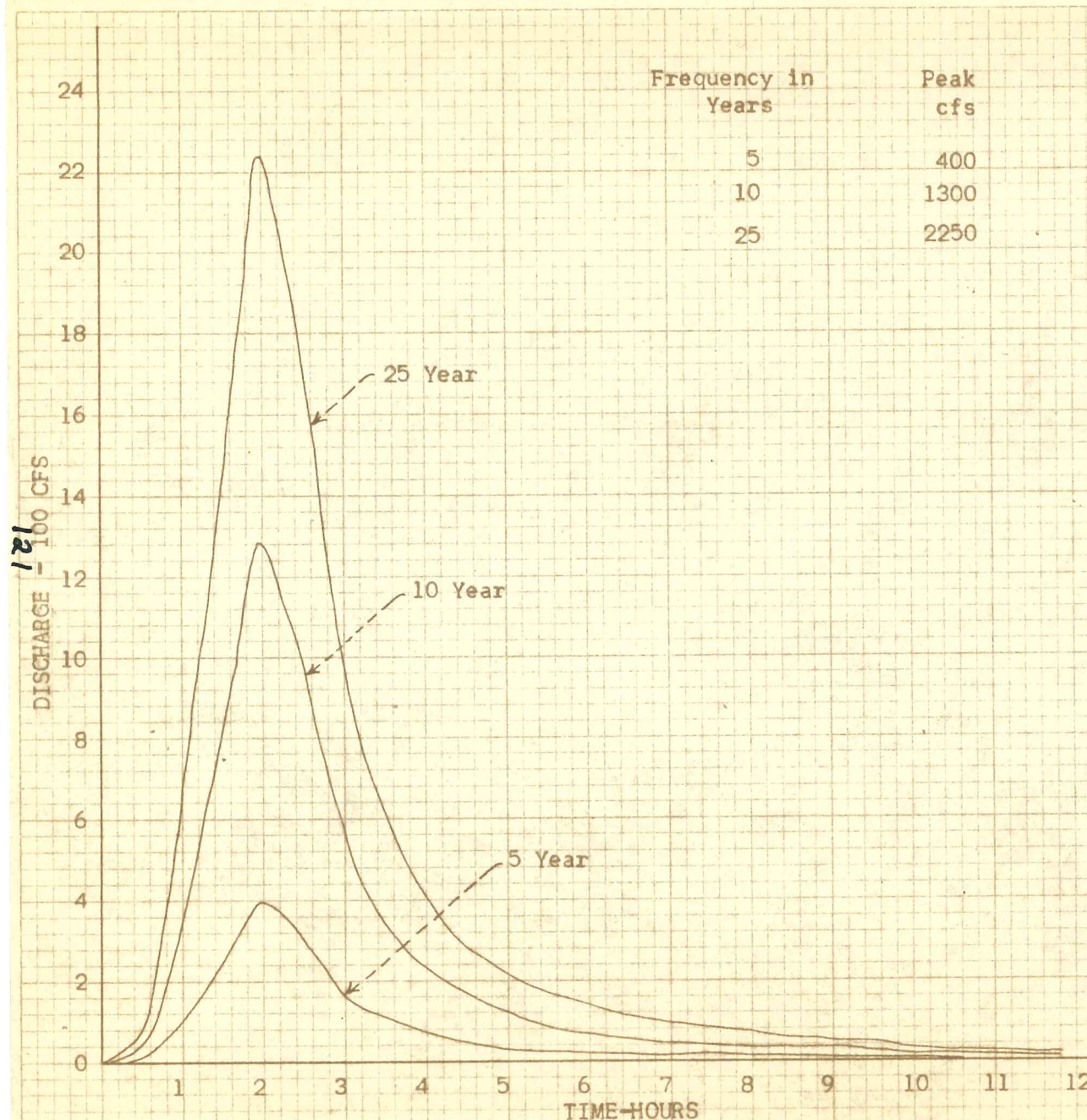
STANAKER DAM SITE

INFLOW DESIGN FLOOD

Drawn by D.L.M.  
6-6-56

Traced by L.M.N.  
Denver, Colo.





United States  
Department of the Interior  
Bureau of Reclamation  
DIVISION OF PROJECT INVESTIGATIONS  
Central Utah Project - Vernal Unit

STANAKER DAM SITE  
DIVERSION REQUIREMENT HYDROGRAPHS  
(SUMMER SEASON)

Drawn by D.L.M.  
5/7/56

Traced by L.M.N.  
Denver, Colo.